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**THERMAL STRAIN ANALYSIS OF
ADVANCED MANNED SPACECRAFT HEAT SHIELDS**

Final Report

To

NATIONAL AERONAUTICS AND SPACE ADMINISTRATION
MANNED SPACECRAFT CENTER
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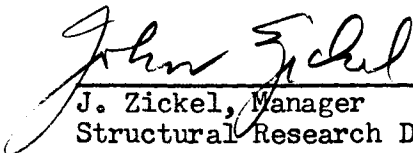
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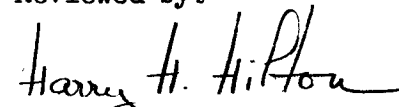
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ABSTRACT

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Numerical methods and computer programs are presented for the analysis of heat shields. The finite element technique is used to determine stresses and displacements developed in composite axisymmetric solids of arbitrary geometry subjected to axisymmetric thermal or mechanical loads. This technique is then applied to the development of an automated computer program for the analysis of axisymmetric heat shields subjected to axisymmetric thermal and pressure loadings. Finally, the numerical technique is extended to the analysis of heat shields subjected to non-axisymmetric thermal loading.

Several examples are presented to illustrate the application of the method and to demonstrate its validity. FORTRAN II card listings and descriptions of the use of the above programs are given in the appendices.

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LIST OF SYMBOLS

a_j, b_j, a_k, b_k = Element Dimensions

E = Modulus of Elasticity

u = Displacement in the r Direction

v = Displacement in the θ Direction

w = Displacement in the z Direction

α = Thermal Coefficient of Expansion

β = Over-Relaxation Factor

γ = Shear Strain

$\epsilon_{r,\theta,z}$ = Strain in the Radial, Circumferential and Longitudinal Direction

ν = Poisson's Ratio

$\sigma_{r,\theta,z}$ = Stress in the Radial, Circumferential and Longitudinal Direction

τ = Shearing Stress

$[]^T$ = Matrix Transpose

$[a]$ = Displacement Transformation Matrix

$[C]$ = Matrix of Elastic Coefficients

$[G_n]$ = Displacement Transformation Matrix - Harmonic n

$[S]$ = Matrix of Element Corner Forces

$[u]$ = Matrix of Element Corner Displacements

$[k]$ = Element Stiffness Matrix

$[F]$ = Nodal Point Displacements

$[R]$ = Nodal Point Loads

$[K]$ = Stiffness Matrix for Complete Structure

INTRODUCTION

The purpose of this investigation is the development of methods of analysis and digital computer programs to aid in establishing the structural integrity of manned spacecraft heat shields. The results of the analysis which are presented in this report indicate only the capabilities of the computer programs and do not necessarily represent the behavior of a specific heat shield. The final evaluation of the structural capability of a heat shield must be based on a certain amount of engineering judgement, in connection with the use of the computer programs.

In this investigation the finite element method is used to determine stresses and displacements developed in solids of revolution. First, a numerical procedure and a digital computer program are developed for the analysis of composite axisymmetric solids of arbitrary geometry subjected to axisymmetric thermal or mechanical loads. Second, this program is specialized to the analysis of axisymmetric heat shields. Finally, the same numerical technique is extended to the analysis of heat shields subjected to non-axisymmetric thermal loading. A description of the method of analysis and the use of the above computer programs is presented. In addition, FORTRAN II listings of the above programs are incorporated in this report.

During the initial phases of this contract, finite difference techniques were used to solve the governing differential equations for displacements of the system. However, considerable difficulty was encountered in the solution of the resulting set of linear equations. An iterative approach, coupled with over-relaxation techniques, resulted in inadequately convergent displacements. The direct solution technique gave a matrix for the set of simultaneous equations which was ill-conditioned. An additional difficulty of the finite difference technique was encountered in satisfying the boundary conditions at the edge of the heat shield. The finite element approach proved more practical and more versatile; therefore, the finite difference method was discontinued.

PART I: METHOD OF ANALYSIS

A. INTRODUCTION

The "finite element method" is a general method of structural analysis in which a continuous structure is replaced by a finite number of elements interconnected at a finite number of nodal points -- (such an idealization is inherent in the conventional analysis of frames and trusses). In this investigation the finite element method is applied to the determination of stresses and displacements developed in axisymmetric elastic structures of arbitrary geometry and material properties which are subjected to thermal and mechanical loads.

An assemblage of different types of axisymmetric elements is used to represent the continuous structure. Approximations are made on the displacements within each element of the system. Based on these approximations, equilibrium equations are developed for all elements. From "direct stiffness techniques", the equilibrium equations, in terms of unknown nodal point displacements, are developed at each nodal point. A solution of this set of equations constitutes a solution to the finite element system.

B. EQUILIBRIUM EQUATIONS FOR AN ARBITRARY FINITE ELEMENT

1. Strain-Displacement Relationship

The first step in the determination of the stiffness (corner forces in terms of corner displacements and temperature changes), of a

finite element is to assume a solution for the displacement field within the element. It is desirable that this assumed displacement field satisfies compatibility between other elements in the system. Based on this solution for the displacements within the element, it is possible to develop an expression for the strains at any point within the element in terms of the nodal points (corner) displacements. This expression in matrix form is

$$[\epsilon] = [a][u] \quad (1.1)$$

where $[\epsilon]$ is a column matrix of the M components of strain
 $[u]$ is a column matrix of the N nodal point displacements
 $[a]$ is an M x N strain-displacement transformation matrix - this matrix may be a function of space

2. Stress-Strain Relationship

For an elastic material, the stresses at any point within the element are expressed in terms of the corresponding strains by the elastic stress-strain relationship. Or, in matrix form

$$[\sigma] = [C][\epsilon] + [\tau] \quad (1.2)$$

where $\begin{bmatrix} \sigma \end{bmatrix}$ is a column matrix of the M components of stress
 $\begin{bmatrix} \epsilon \end{bmatrix}$ is a column matrix of the M components of strain
 $\begin{bmatrix} \tau \end{bmatrix}$ is a column matrix of the M components of thermal stress
 $\begin{bmatrix} C \end{bmatrix}$ is an MxM matrix of material property coefficients

The size (M) of these matrices will depend on the type of element being considered. The coefficients of matrices $\begin{bmatrix} C \end{bmatrix}$ and $\begin{bmatrix} \tau \end{bmatrix}$ will depend on material properties. Since $\begin{bmatrix} C \end{bmatrix}$ is completely arbitrary, anisotropic materials can be handled. Also, each element in the system may have different properties; therefore, composite structures are readily represented by the finite element idealization.

3. Internal Work

The internal work, or strain energy, which is associated with an infinitesimal volume element dV within the finite element is given by

$$dW_I = \frac{1}{2} (\epsilon_1 \sigma_1 + \epsilon_2 \sigma_2 + \dots + \epsilon_M \sigma_M) dV$$

or in matrix form

$$dW_I = \frac{1}{2} \begin{bmatrix} \epsilon \end{bmatrix}^T \begin{bmatrix} \sigma \end{bmatrix} dV \quad (1.3)$$

The substitution of Equation (1.2) into Equation (1.3) yields

$$dW_I = \frac{1}{2} \begin{bmatrix} \epsilon \end{bmatrix}^T \begin{bmatrix} C \end{bmatrix} \begin{bmatrix} \epsilon \end{bmatrix} dV + \frac{1}{2} \begin{bmatrix} \epsilon \end{bmatrix}^T \begin{bmatrix} \tau \end{bmatrix} dV \quad (1.4)$$

Equation (1.1) may be written in transposed form as

$$[\epsilon]^T = [u]^T [a]^T \quad (1.5)$$

After Equations (1.1) and (1.5) are substituted into Equation (1.4), the internal work is given by

$$dW_I = \frac{1}{2} [u]^T [a]^T [C] [a] [u] dV + \frac{1}{2} [u]^T [a]^T [\tau] dV \quad (1.6)$$

The total strain energy stored within the element is found by integrating Equation (1.6) over the volume of the finite element. Or

$$W_I = \frac{1}{2} [u]^T \int [a]^T [C] [a] [u] dV + \frac{1}{2} [u]^T \int [a]^T [\tau] dV \quad (1.7)$$

4. External Work

The work supplied externally at the nodal points of the finite element is given by

$$W_E = \frac{1}{2} U_1 S_1 + \frac{1}{2} U_2 S_2 \dots\dots\dots + \frac{1}{2} U_N S_N$$

or in matrix form

$$W_E = \frac{1}{2} [u]^T [S] \quad (1.8)$$

where $[u]^T$ is a row matrix of the N nodal point displacements

$[S]$ is a column matrix of the N corresponding nodal point forces

5. External Work = Internal Work

The external work, Equation (1.8), is equated to the internal work, Equation (1.7), yielding

$$[u]^T [S] = [u]^T [k] [u] + [u]^T [L] \quad (1.9)$$

where the element stiffness matrix

$$[k] = \int [a]^T [C] [a] dV \quad (1.10)$$

and the thermal load matrix

$$[L] = \int [a]^T [\tau] dV \quad (1.11)$$

Equation (1.9) represents an energy balance (scalar equation) for a single nodal point displacement pattern. If the final displacements $[u_i]$ are assumed to be composed of N separate displacement patterns, $[\bar{u}_{ij}]$ $j = 1, \dots, N$, and if the final forces $[S_i]$ are assumed to be composed of N corresponding sets of forces, $[\bar{S}_{ij}]$ $j = 1, \dots, N$, Equation (1.9) may be written as

$$[\bar{u}]^T [\bar{S}] = [\bar{u}]^T [k] [\bar{u}] + [\bar{u}]^T [L] \quad (1.12)$$

To eliminate the term $[\bar{u}]^T$, the displacement patterns must be selected in such a manner as to assure an inverse of $[\bar{u}]^T$. An acceptable matrix is a diagonal matrix of the final displacement, or

$$[\bar{u}]^T = [\bar{u}] = [u] \quad (1.13)$$

Equation (1.12) is now premultiplied by $[u]^{-1}$ yielding

$$[I][S] = [I][k][u] + [I][L] \quad (1.14)$$

where $[I]$ is a diagonal unit matrix.

Since only linear systems are considered, the N displacement patterns may be superimposed. Or

$$[S] = [k][u] + [L] \quad (1.15)$$

Since

$$s_i = \sum_{j=1, \dots, N} \bar{s}_{ij} \quad \text{and} \quad u_i = \bar{u}_{ii}$$

Equation (1.15) expresses nodal point forces in terms of nodal point displacements and temperature changes within the element.

C. EQUILIBRIUM EQUATIONS FOR A SYSTEM OF FINITE ELEMENTS

The first step in the procedure is to express all element forces in terms of external nodal point displacements for each element in the system. This is accomplished by expanding Equation (1.14) in terms of the N possible nodal point displacements; this will yield M matrix equations of the form

$$[S^m] = [k^m][r] + [L^m] \quad m = 1, \dots, M \quad (1.16)$$

where M is the total number of elements in the system.

The matrix $[k^m]$ is termed the complete stiffness of element m and involves only terms which are associated with the displacements of the connecting nodal points. Consequently, the majority of the coefficients of this matrix equation are zero. The matrix $[r]$ contains all possible nodal point displacements of the complete finite element system. The matrix $[S^m]$ is a column matrix containing the forces acting on element m in the direction of the nodal point displacements $[r]$. The thermal load matrix $[L^m]$ and the element stiffness matrix $[k^m]$ are given by Equations (1.11) and (1.10); however, the order (size) of these matrices has now been expanded to correspond with the total number of nodal point displacements.

In order to satisfy equilibrium of all nodal points, the sum of the internal element forces must be equal to the external nodal point loads.

Or

$$[P] = \sum_{m=1, \dots, M} [S^m] \quad (1.17)$$

where $[P]$ is the externally applied nodal point loads. The substitution of Equation (1.16) into Equation (1.17) yields

$$[P] = \sum_{m=1, \dots, M} [k^m][r] + \sum_{m=1, \dots, M} [L^m] \quad (1.18)$$

or rewritten in the following form:

$$[R] = [K][r] \quad (1.19)$$

where

$$[R] = [P] - \sum_{m=1, \dots, M} [L^m] \quad (1.20)$$

$$[K] = \sum_{m=1, \dots, M} [k^m] \quad (1.21)$$

Equation (1.19), which is an equilibrium relationship between external loads and internal forces, represents a system of N linear equations in terms of N unknown displacements.

D. SOLUTION OF EQUILIBRIUM EQUATIONS

Equation (1.19) represents the relationship between all nodal point forces and all nodal point displacements. Mixed boundary conditions are considered by rewriting Equation (1.19) in the following partitioned form:

$$\begin{bmatrix} R_a \\ \text{---} \\ R_b \end{bmatrix} = \begin{bmatrix} K_{aa} & K_{ab} \\ \text{---} & \text{---} \\ K_{ba} & K_{bb} \end{bmatrix} \begin{bmatrix} r_a \\ \text{---} \\ r_b \end{bmatrix} \quad (1.22)$$

where $[R_a]$ = the specified nodal point forces
 $[R_b]$ = the unknown nodal point forces
 $[r_a]$ = the unknown nodal point displacements
 $[r_b]$ = the specified nodal point displacements

Equation (1.22) may be expressed in terms of two separate equations, or

$$[R_a] = [K_{aa}][r_a] + [K_{ab}][r_b] \quad (1.23)$$

$$[R_b] = [K_{ba}][r_a] + [K_{bb}][r_b] \quad (1.24)$$

Equation (1.23) is rewritten in the following reduced form:

$$[K_{aa}][r_a] = [\bar{R}_a] \quad (1.25)$$

where the modified load vector, $[\bar{R}_a]$ is given by

$$[\bar{R}_a] = [R_a] - [K_{ab}][r_b] \quad (1.26)$$

In Part II of this report, the Gauss-Seidel iterative technique is used to solve Equation (1.25) for the unknown nodal point displacements $[r_a]$. Appendix A gives a direct solution approach which is used in the automated computer programs for the thermal stress analysis of heat shields, Parts III and IV of this report.

E. ELEMENT STRESSES

After the nodal point displacements have been determined, the strains within any element in the system are evaluated by the direct application of Equation (1.1). The corresponding stresses are calculated from the stress-strain relationship, Equation (1.2).

PART II GENERAL COMPUTER PROGRAM FOR THE ANALYSIS
OF ARBITRARY AXISYMMETRIC STRUCTURES

A. INTRODUCTION

The stress analysis of an axisymmetric structure of arbitrary shape, subjected to thermal and mechanical loads is of considerable practical interest. Although the governing differential equations have been known for many years, closed form solutions have been obtained for only a limited number of structures. Thus, the investigator must often rely on experimental or numerical procedures to solve this problem.

Experimental methods, such as Photoelasticity, have proven to be versatile tools in the analysis of many axisymmetric structures. However, for structures composed of several different materials or structures with thermal loading, this approach is limited.

The finite difference method, which involves the replacement of the derivatives in the differential equations and boundary conditions with difference equations, has been the most popular of the numerical techniques. However, for structures of composite materials and of arbitrary geometry, the procedure is difficult to apply.

In this section the finite element method is used to determine the stresses and displacements developed within arbitrary, elastic solids of revolution subjected to thermal or mechanical axisymmetric loads. The

finite element approach replaces the continuous structure with a system of triangular rings interconnected at a finite number of nodal points (joints). Loads acting on the structure are replaced by statically equivalent concentrated forces acting at the nodal points of the finite element system. Figure 2.1 illustrates a finite element idealization of a typical axisymmetric solid.

B. STIFFNESS OF TRIANGULAR RING

1. Strain-Displacement Relationship

Continuity between elements of the system is maintained by requiring that within each element "lines initially straight remain straight in their displaced position". This linear displacement field, which is illustrated in Figure 2.2, is defined in terms of $u(r,z)$ and

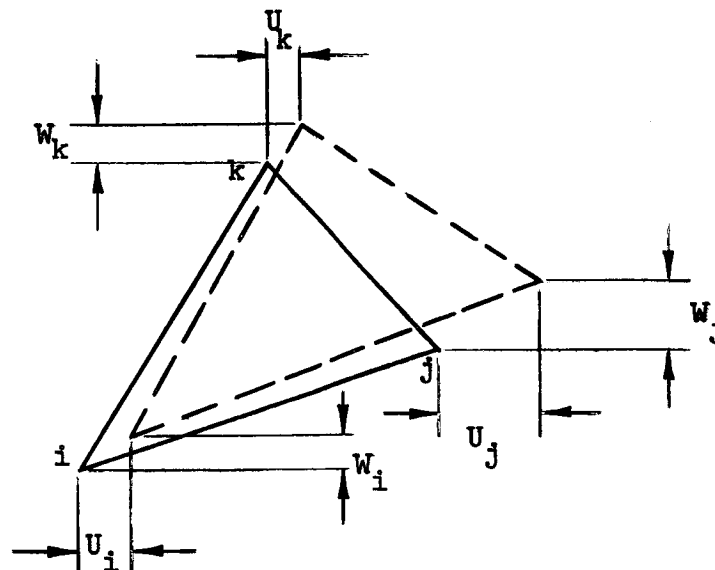
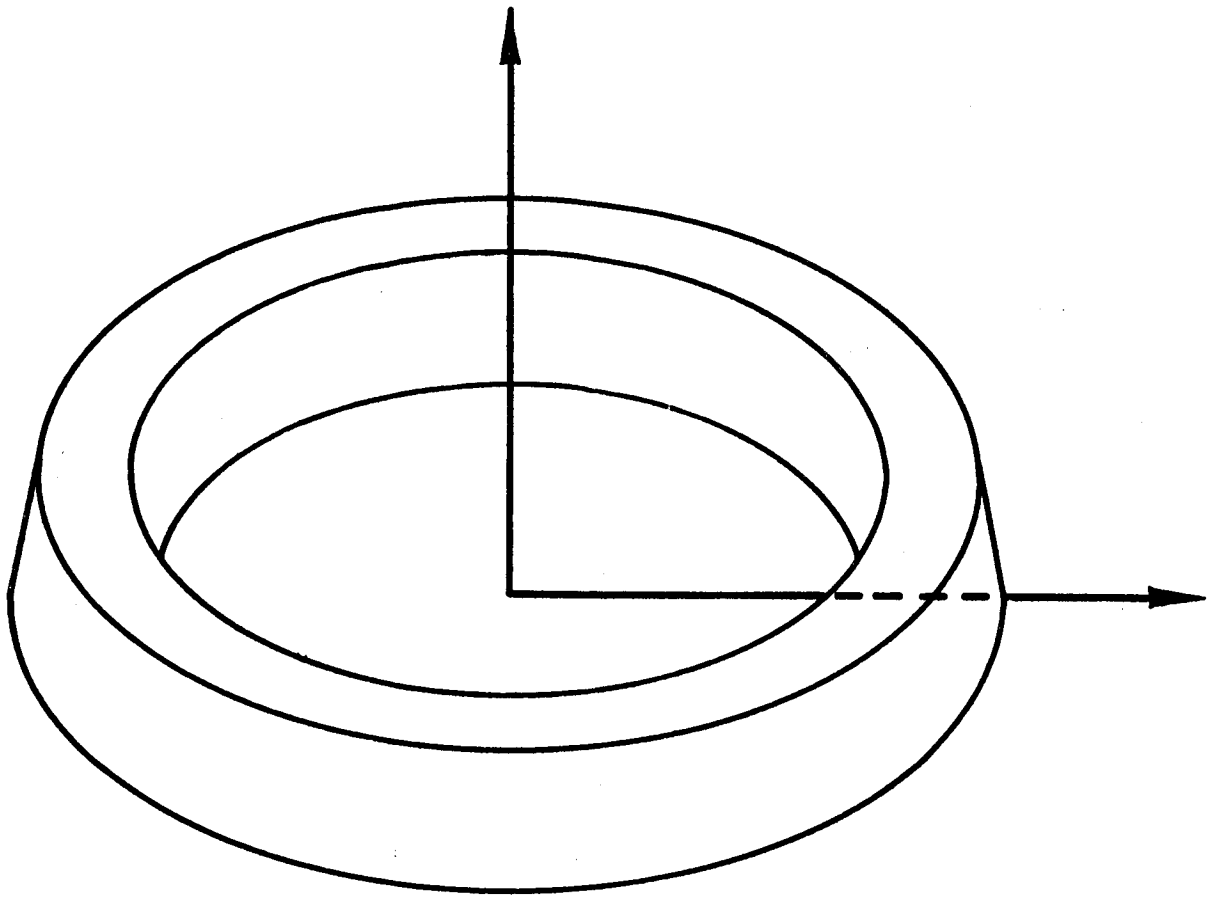
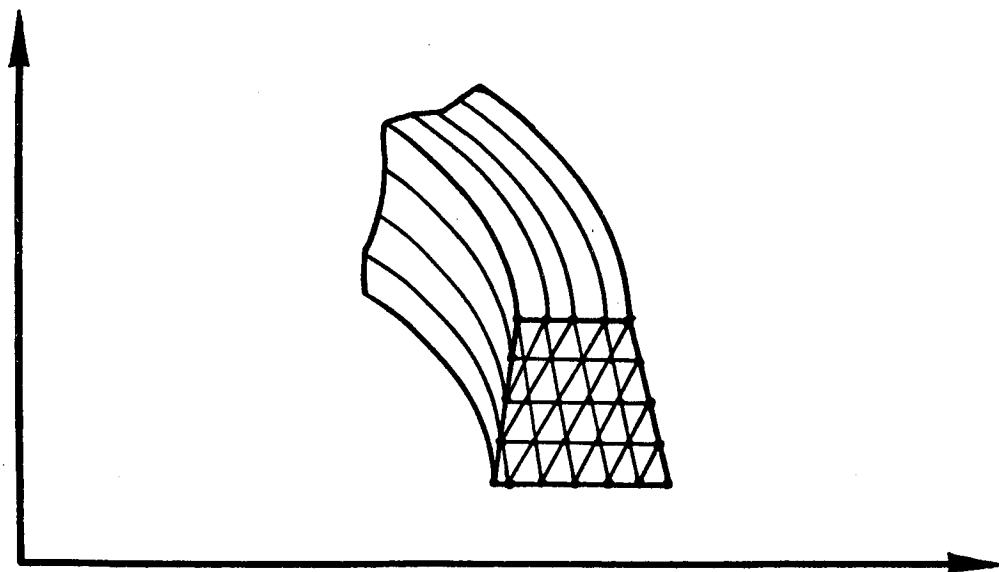


FIG. 2.2 ASSUMED DISPLACEMENT PATTERN



a. ACTUAL RING



b. TRIANGULAR ELEMENT APPROXIMATION

FIG. 2.1 THE FINITE ELEMENT IDEALIZATION

$w(r,z)$ by equations of the following form:

$$u(r,z) = C_1 + C_2 r + C_3 z \quad (2.1a)$$

$$w(r,z) = C_4 + C_5 r + C_6 z \quad (2.1b)$$

If Equations (2.1a) and (2.1b) are evaluated at the three corners i, j, k of the triangle, the following set of equations is obtained:

$$\begin{bmatrix} u_i \\ w_i \\ u_j \\ w_j \\ u_k \\ w_k \end{bmatrix} = \begin{bmatrix} 1 & r_i & z_i & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r_i & z_i \\ 1 & r_j & z_j & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r_j & z_j \\ 1 & r_k & z_k & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r_k & z_k \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_5 \\ C_6 \end{bmatrix} \quad (2.2)$$

By solving the system of Equations (2.2) for the constants C_1, \dots, C_6 , they are expressed in terms of corner displacements. The strains in the rz -plane are obtained from the assumed displacement field by considering the basic definition of strain.

$$\bar{\epsilon}_r = \frac{\partial u}{\partial r} = C_2 \quad (2.3a)$$

$$\bar{\epsilon}_z = \frac{\partial w}{\partial z} = C_6 \quad (2.3b)$$

$$\bar{\gamma}_{rz} = \frac{\partial u}{\partial z} + \frac{\partial w}{\partial r} = C_3 + C_5 \quad (2.3c)$$

At any point within the element the tangential strain $\bar{\epsilon}_\theta$ is

$$\bar{\epsilon}_\theta(r,z) = \frac{u(r,z)}{r}$$

The average tangential strain is found by averaging the strains at the vertices of the triangle, or

$$\epsilon_\theta = \frac{1}{3} \left(\frac{u_i}{r_i} + \frac{u_j}{r_j} + \frac{u_k}{r_k} \right) \quad (2.3d)$$

After eliminating the constants C_n between Equations (2.2) and (2.3), the average element strains are expressed in terms of corner displacements by the following matrix equation:

$$\begin{bmatrix} \epsilon_r \\ \epsilon_z \\ \epsilon_\theta \\ \gamma_{rz} \end{bmatrix} = \frac{1}{\lambda} \begin{bmatrix} b_j - b_k & 0 & b_k & 0 & -b_j & 0 \\ 0 & a_k - a_j & 0 & -a_k & 0 & a_j \\ \frac{\lambda}{3r_i} & 0 & \frac{\lambda}{3r_j} & 0 & \frac{\lambda}{3r_k} & 0 \\ a_k - a_j & b_j - b_k & -a_k & b_k & a_j & -b_j \end{bmatrix} \begin{bmatrix} u_i \\ w_i \\ u_j \\ w_j \\ u_k \\ w_k \end{bmatrix} \quad (2.4a)$$

or in symbolic form

$$[\epsilon] = [a][u] \quad (2.4b)$$

where

$$a_j = r_j - r_i$$

$$a_k = r_k - r_i$$

$$b_j = z_j - z_i$$

$$b_k = z_k - z_i$$

$$\lambda = a_j b_k - a_k b_j$$

The geometry of a typical triangle is illustrated in Figure 2.3

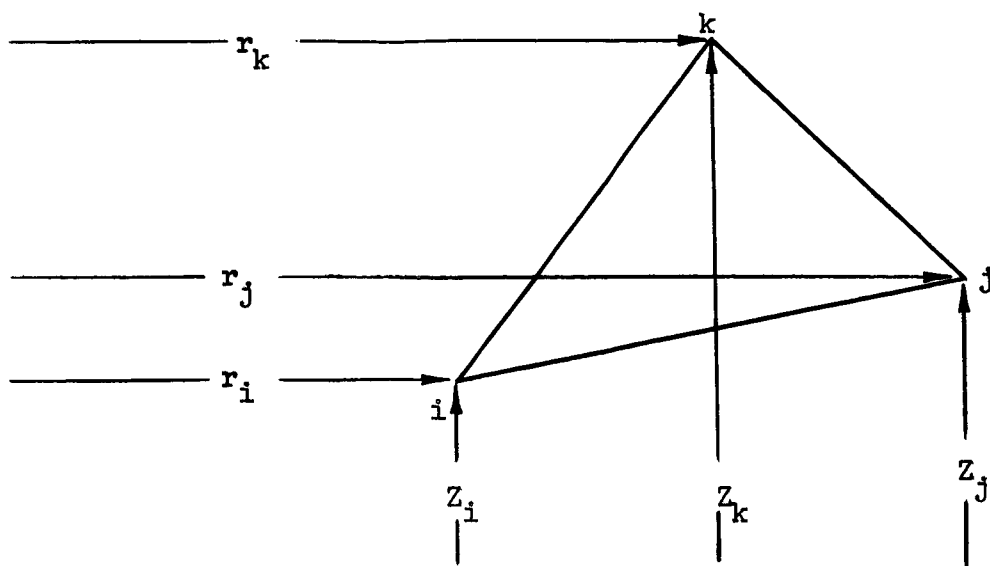


FIG. 2.3 ELEMENT DIMENSIONS

2. Stress-Strain Relationship

One important advantage of the finite element approach is that structures with anisotropic materials can be treated. In general, the stress-strain relationship is of the form

$$\begin{bmatrix} \sigma_r \\ \sigma_z \\ \sigma_\theta \\ \sigma_{rz} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} \\ C_{21} & C_{22} & C_{23} & C_{24} \\ C_{31} & C_{32} & C_{33} & C_{34} \\ C_{41} & C_{42} & C_{43} & C_{44} \end{bmatrix} \begin{bmatrix} \epsilon_r \\ \epsilon_z \\ \epsilon_\theta \\ \gamma_{rz} \end{bmatrix} + \begin{bmatrix} \tau_r \\ \tau_z \\ \tau_\theta \\ \tau_{rz} \end{bmatrix} \quad (2.5a)$$

$$\text{or symbolically } [\sigma] = [C] [\epsilon] + [\tau] \quad (2.5b)$$

where $[\tau]$ is the matrix of thermal stresses for a given temperature change. For example, the stress-strain relationship for an isotropic material is given by

$$\begin{bmatrix} \sigma_r \\ \sigma_z \\ \sigma_\theta \\ \sigma_{rz} \end{bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & \nu & 0 \\ \nu & 1-\nu & \nu & 0 \\ \nu & \nu & 1-\nu & 0 \\ 0 & 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{bmatrix} \epsilon_r \\ \epsilon_z \\ \epsilon_\theta \\ \gamma_{rz} \end{bmatrix} + \begin{bmatrix} \tau \\ \tau \\ \tau \\ 0 \end{bmatrix} \quad (2.6)$$

$$\text{where } \tau = \frac{E\alpha}{(1-2\nu)} \Delta T \quad (2.7)$$

3. Element Stiffness

The stiffness of a typical triangular ring, which is an expression for corner forces in terms of corner displacements, is given by Eq. (1.10) as

$$[k] = \int [a]^T [c] [a] dV$$

And the thermal load matrix is given by Eq. (1.11) as

$$[L] = \int [a]^T [\tau] dV$$

Since the coefficients in matrices $[a]$ and $[c]$ are assumed not to be a function of space the above equations reduce to

$$[k] = V [a]^T [C] [a] \quad (2.8)$$

$$[L] = V [a]^T [\tau] \quad (2.9)$$

If a one-radian segment is considered, an approximate expression for the volume of a triangular ring segment is

$$V = \bar{r} A \quad (2.10)$$

where \bar{r} is the average radius given by

$$\bar{r} = (r_i + r_j + r_k)/3 \quad (2.11)$$

and A is the cross-sectional area given by

$$A = (a_j b_k - a_k b_j)/2 \quad (2.12)$$

From Eq. (1.15) the six corner forces acting at the vertices of a one-radian triangular segment is given in terms of the six corner displacements and the temperature change within the element by

$$[S] = [k] [u] + [L] \quad (2.13)$$

C. EQUILIBRIUM EQUATIONS FOR COMPLETE STRUCTURE

The equilibrium of the complete system of triangular rings, which is an expression for nodal point loads in terms of nodal point displacements, is given by the following matrix equation:

$$[R] = [K] [r] \quad (2.14)$$

The stiffness matrix $[K]$ and the load matrix $[R]$ are determined by "direct stiffness" techniques as indicated in the previous section, Eqs. (1.20) and (1.21). In addition to the thermal loads, the $[R]$ matrix is composed of concentrated external forces acting at the nodal points of the system. Hence, pressures acting on the boundary of a segment of the structure are replaced by statically equivalent forces acting at the nodal points.

Mixed boundary conditions are considered by a simple transformation of Eq. (2.14); Eqs. (1.22) to (1.26) give the details of this modification.

D. DETERMINATION OF DISPLACEMENT AND STRESSES

Equation (2.14) is solved for the unknown nodal point displacements by the application of the well-known Gauss-Seidel iterative procedure. This involves the repeated calculation of new displacements from the equation

$$r_n^{(s)} = K_{nn}^{-1} \left[R_n - \sum_{i=1, \dots, n-1} K_{ni} r_i^{(s)} - \sum_{i=n+1, \dots, n} K_{ni} r_i^{(s-1)} \right] \quad (2.15)$$

where n is the number of the unknown and s is the cycle of iteration.

The only modification of the procedure introduced in this analysis is the simultaneous application of Equation (2.15) to both components

of displacements at each nodal point. Therefore, r_n and R_n become vectors with r and z components.

The rate of convergence of the Gauss-Seidel procedure can be greatly increased by the use of an over-relaxation factor. This factor is applied by first calculating the change in displacement $\Delta r_n^{(s)}$ of nodal point n and then determining the new displacement from the following equation:

$$r_n^{(s)} = r_n^{(s-1)} + \beta \Delta r_n^{(s)} \quad (2.16)$$

where β is the over-relaxation factor.

The solution of an over-relaxation factor, which gives the best convergence, depends on the characteristics of the particular problem. However, experience has indicated that for most structures, the optimum over-relaxation factor is between 1.8 and 1.95.

Since only the non-zero terms in Equation (2.14) are developed and stored by the computer program, a solution of several hundred equations is possible, thereby, making it possible to solve large finite element systems.

For each element the average strains are calculated directly from the nodal point displacements by the application of Equation (2.4). The

average element stresses are then determined from the stress-strain relationship for the element, Equation (2.5). In addition, at each nodal point, stresses are computed by averaging the stresses in all elements attached to the point.

E. COMPUTER PROGRAM

The complete analysis of an axisymmetric solid by the finite element method involves three separate phases. First, the structure must be idealized by a system of triangular rings. Second, this system is solved for displacements and stresses from given nodal point forces. Third, the displacements and stresses are presented graphically for further evaluation and utilization.

The selection of the system of finite elements for a particular problem is completely arbitrary; therefore, axisymmetric structures, composed of many interacting components, of practically any shape may be handled. By numbering all elements and nodal points, in a convenient manner, the system can be defined in the form of three numerical arrays - nodal point array, element array and boundary point array. The nodal point array contains the coordinates and the loads or displacements that are associated with each nodal point of the system. The element array contains, for each element in the system, the location of the element (the three nodal point numbers defining the corners of the element and other possible parameters which are associated with the element (i.e.,

elastic constants, density and temperature changes). The boundary array indicates the type of restraint that exists at boundary nodal points.

These three arrays, along with some basic control information, constitute the numerical input for the digital computer program. The program itself performs three major tasks in the analysis of the finite element system of triangular rings. First, the equilibrium equations for the system are formed from the basic numerical description of the system. Second, this set of equations is solved for the nodal point displacements. Third, the internal stresses are determined from these displacements.

1. Input Information

To define the system of finite elements, all nodal points and elements are numbered as illustrated in Figure 2.4. Based on this numbering system, the following sequence of punched cards constitutes the input to the computer program.

a. TITLE CARD (72H)

Columns 2 to 72 of this card contain information to be printed with results

b. CONTROL CARD (6I4, 2E12.5)

Columns 1 - 4 Number of elements
5 - 8 Number of nodal points
9 - 12 Number of restrained boundary points
13 - 16 Cycle interval for print of unbalanced forces

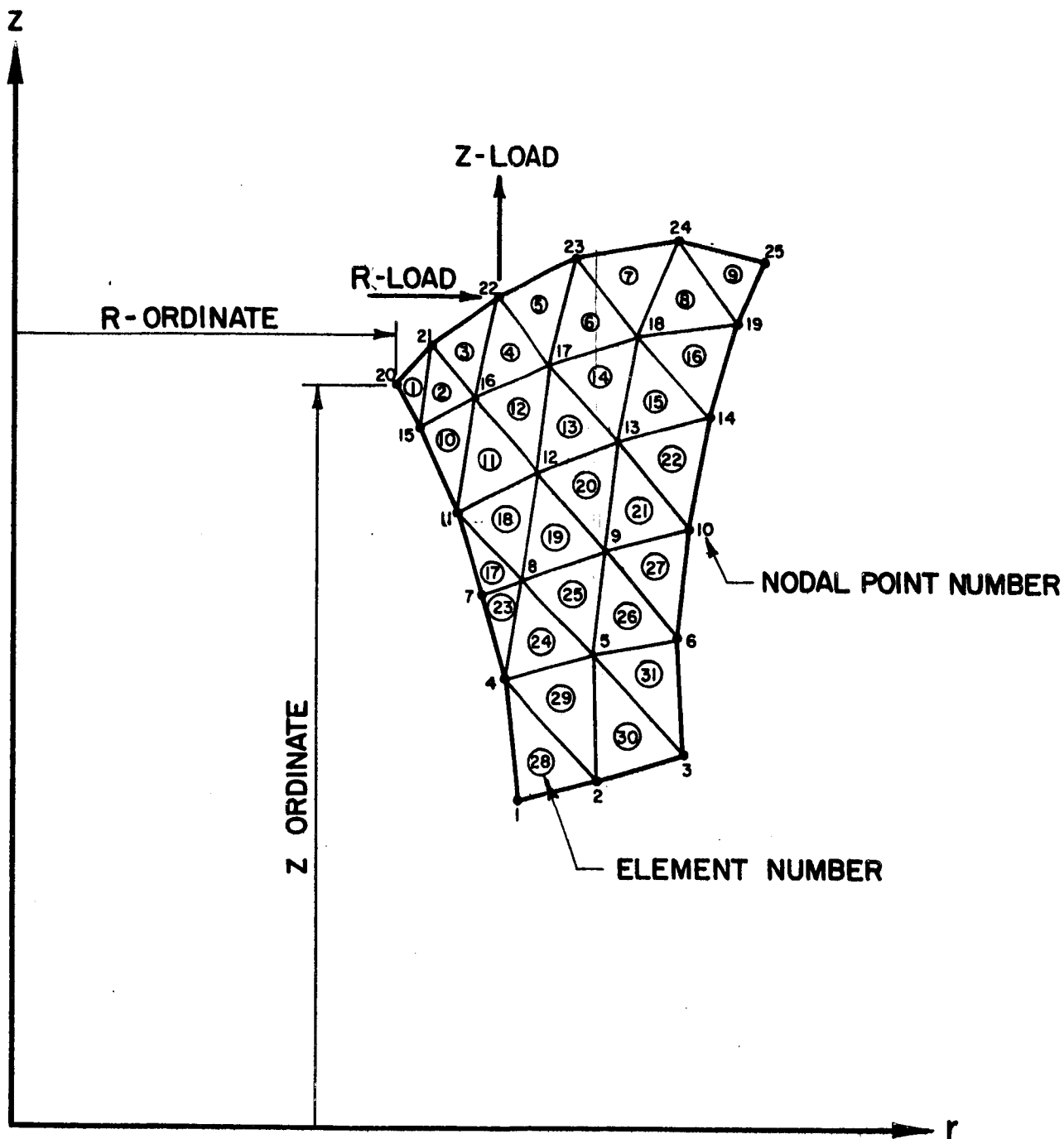


FIG. 2.4 NUMBERING SYSTEM FOR ELEMENTS AND NODAL POINTS

17-20 Cycle interval for print of results
 21-24 Maximum number of cycles problem may
 run
 25-36 Convergence limit for unbalanced forces
 37-48 Over-relaxation factor

c. ELEMENT ARRAY - 1 card per element (4I4, 4E12.4,
 F8.4)

Column	1-4	Element Number	
	5-8	Nodal point number i	
	9-12	Nodal point number j	} in counter- clockwise order
	13-16	Nodal point number k	
	17-28	Modulus of elasticity E	
	29-40	Density of element ρ	
	41-52	Poisson's ratio ν	
	53-64	Coefficient of thermal expansion α	
	65-72	Temperature change within element ΔT	

d. NODAL POINT ARRAY - 1 card per nodal point (1I4,
 4F8.1, 2F12.8)

Column	1-4	Nodal point number	
	5-12	R-ordinate	
	13-20	Z-ordinate	
	21-38	R-load	} Total force acting on a one radian segment.
	29-36	Z-load	
	37-48	R-displacement	
	49-60	Z-displacement	

On free nodal points, the displacements are initial guesses for the
 iterative solution. On restrained nodal points, the input displacements
 are the specified final displacements of the nodal point.

- e. BOUNDARY POINT ARRAY - 1 card per restrained nodal point (2I4, IF6)

Columns 1-4 Nodal point numbers

5-8 0 if point is fixed in both directions

1 if point is fixed in the R-direction

2 if point is free to move along a line of slope S

9-16 Slope S (type 2 points only)

2. Output Information

The following information is generated and printed by the computer program:

- a. Input Data
- b. Nodal Point Displacement
- c. Average Element Stresses
- d. Average Nodal Point Stresses

3. Timing

For the IBM 7094 the computational time required by the program is approximately $0.004 \times n \times m$ seconds, where n equals the number of nodal points and m equals the number of cycles of iteration. Depending on the desired degree of convergence, it may be necessary to extend the iteration process.

4. Program Listing

A card listing of the FORTRAN II source deck for the general axisymmetric program is included in Appendix D of this report. This program is compiled for a maximum size of 550 elements or 340 nodal points.

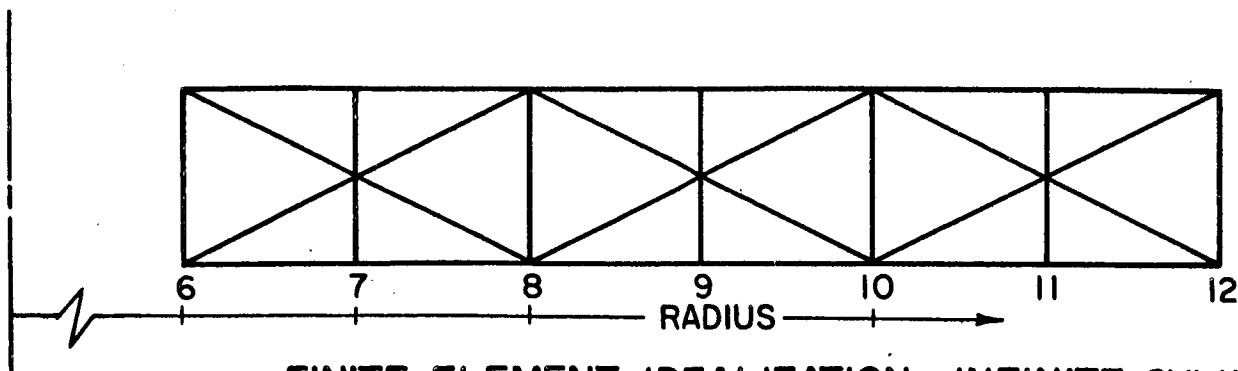
F. EXAMPLE

An infinite cylinder subjected to steady state temperature distribution, for which an exact solution is known, is selected as a means of verifying the finite element analysis. A finite element idealization of a section of the cylinder is shown in Figure 2.5a. The temperature distribution which is assumed constant within each element, is plotted in Figure 2.5b. The hoop stresses are compared with the exact solution in Figure 2.5c. Considering the coarse mesh, agreement with the exact solution is very good except at the two boundary points. This discrepancy is due to the fact that nodal point stresses are calculated by averaging the stresses in the attached elements. Therefore, the boundary nodal point stress reflects the average stress in the elements near the boundary.

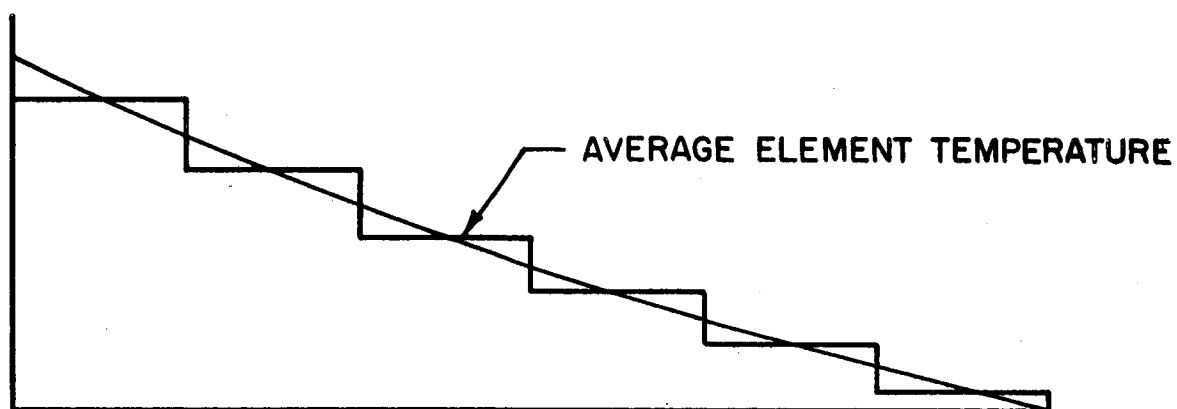
In general, good boundary stresses are obtained by plotting the interior stresses and extrapolating to the boundary. This type of engineering judgement is always necessary in evaluating results from a finite element analysis.

G. DISCUSSION

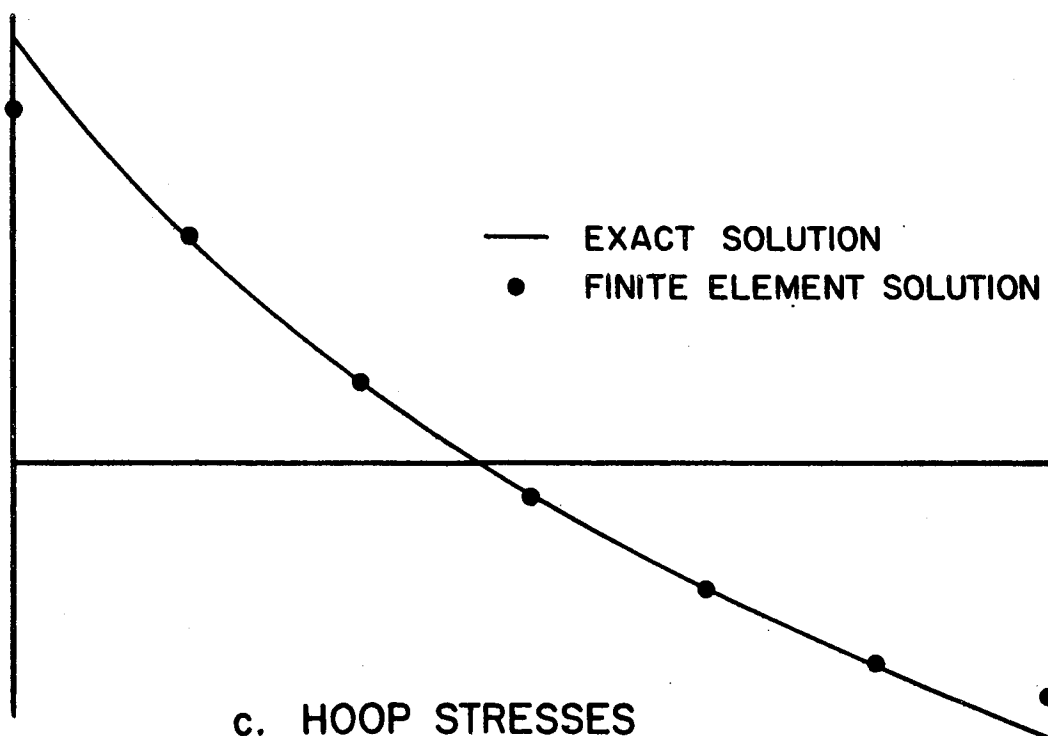
This section demonstrates the application of the finite element technique to the stress analysis of structures of revolution. The approach reduces the stress analysis to a simple procedure. In order to use the



a. FINITE ELEMENT IDEALIZATION - INFINITE CYLINDER



b. TEMPERATURE DISTRIBUTION



c. HOOP STRESSES

FIG. 2.5 ANALYSIS OF INFINITE CYLINDER

program, it is only necessary to select an element idealization of the structure and to supply the computer program with data that numerically define the system of elements. Therefore, the program may be used as a tool in design since changes in materials and geometry of the structures may involve only minor changes in the input data.

In addition, the program may be extended to include the effects of anisotropic materials. For this case, the input to the program must be expanded to include the general elastic constants defined by Eq. (2.5).

For the analysis of a specific type of structure, this program can be further automated by incorporating a mesh generator and the calculation of temperature-dependent material properties. In the next section of this report the program is specialized to the thermal stress analysis of axisymmetric heat shields for manned spacecraft.

PART III AUTOMATED PROGRAM FOR AXISYMMETRIC HEAT
SHIELDS SUBJECTED TO AXISYMMETRIC LOADS

A. INTRODUCTION

The general computer program for the analysis of arbitrary axisymmetric structure, as given in the previous section, can be applied to the thermal stress analysis of heat shields. However, the use of this program for such a complex structure involves a large amount of detail work to select the finite element idealization and to prepare the computer input. In addition, the convergence of the Gauss-Seidel iteration procedure is slow for this type of structure and a solution may require an excessive amount of computer time.

By restricting the general computer program to the analysis of heat shields and by automating the input, a considerably more efficient program can be developed. The geometry of the heat shield is supplied to the computer program in the form of R and Z coordinates and ablator thickness at various points along the bond line. The required triangular mesh and the temperature at the grid points are generated within the program. Material properties at various temperatures are supplied in tabular form and the program automatically develops analytical expressions for the material properties by least square techniques. The flanges of the sandwich shell are idealized by special conical shell elements and the honeycomb core material is treated as a separate material. This approach

eliminates the need for the establishing of a pseudo-thickness for the composite sandwich shell. The solution of the equilibrium equations, which was previously obtained by an iterative approach is accomplished by a direct solution procedure. Because of their significance, stresses within the sandwich plates and at the bond line are included in the computer output.

B. MESH GENERATION

A typical finite element idealization of the cross-section of a heat shield is shown in Figure 3.1. The basic element in this system is a quadrilateral ring, which in turn is composed of two triangular rings (Part II). In this particular case, the sandwich shell is represented by the first two rows of elements and the ablator is idealized by four rows of elements; there are 30 points in the meridional direction. The specific mesh configuration is a variable which is supplied to the computer program.

In general, the geometry of the shell is given by the R-Z coordinates of the points at the bond line between the sandwich shell and the ablator. The points on lines perpendicular to the bond line inside the variable thickness ablator and inside the constant thickness sandwich shell are generated automatically within the program. Thin shell cone elements are used to represent the face plates of the sandwich shell.

From a given temperature distribution at the bond line, the grid point temperatures are assumed to be constant within the shell and are assumed to vary parabolically within the ablator.

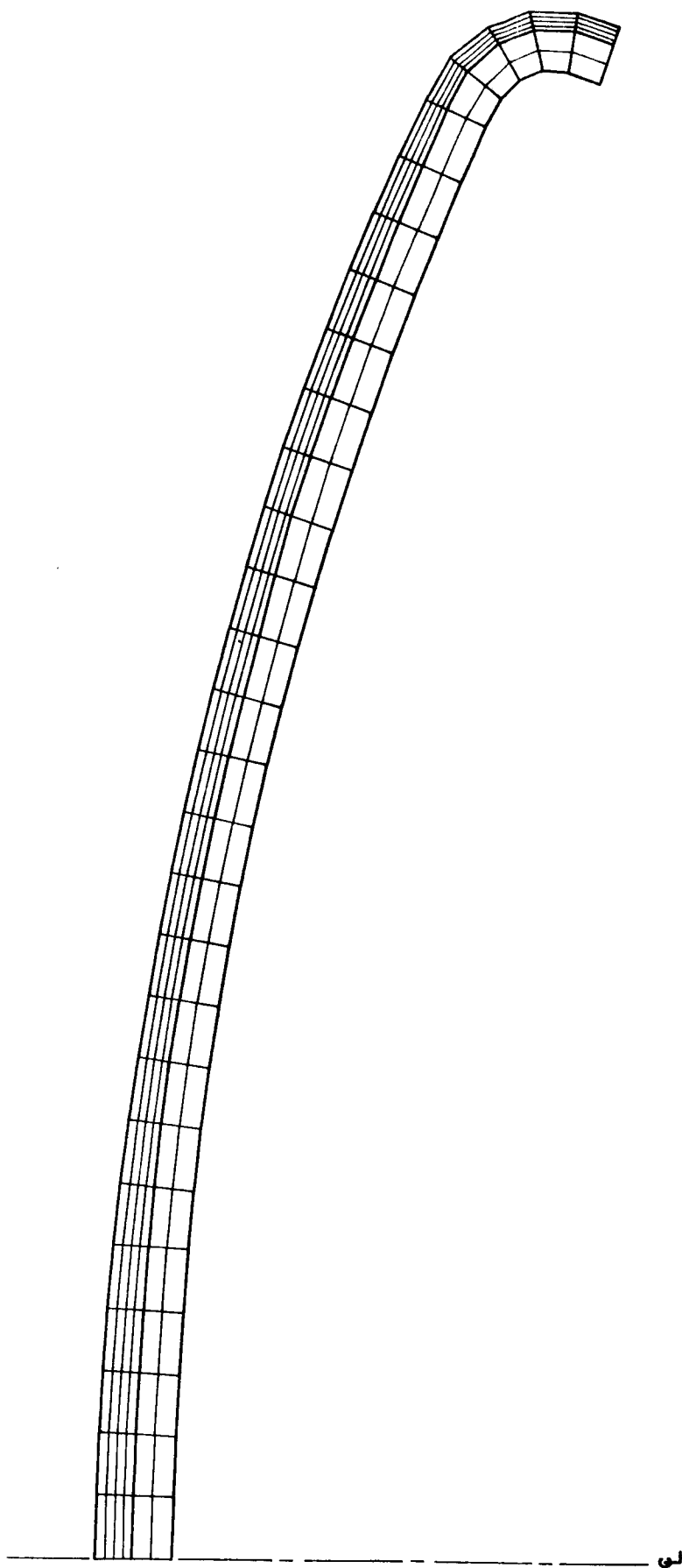


Fig. 3.1 Typical Finite Element Idealization of Heat Shield

C. DETERMINATION OF DISPLACEMENTS AND STRESSES

Based on temperature dependent material properties, the equilibrium relationship for each quadrilateral ring is developed and then combined to form the equilibrium equations of the complete system of rings. Similarly the stiffness properties of the face plates of the sandwich shell are incorporated into the equilibrium of the system. The axisymmetric behavior of a typical conical element is a special case of the non-axisymmetric behavior which is given in Appendix C. The unknowns in this set of equations are the vertical and radial displacements at each grid point in the system. The satisfying of possible displacement boundary conditions requires that these equations be modified as indicated by Equation (1.25). Because of the physical characteristics of the heat shield, the resulting set of equations is in band form. Appendix A indicates the necessary modification to restrict the standard Gaussian elimination procedure to the solution of symmetrical band systems. This approach results in a definite increase in capacity and speed over the iterative technique and it eliminates the problem of convergence.

After the equilibrium equations are solved for the unknown grid point displacements, the average stresses within each triangular ring are calculated as indicated in Part II of this report. Based on the stresses in the two triangular rings, average quadrilateral stresses are calculated for each quadrilateral ring in the system.

D. COMPUTER PROGRAM

The first step in the stress analysis of an axisymmetric heat shield is to select points at the bond line at regular intervals along the meridian of the shield. A quadrilateral mesh is automatically developed by the program from the R and Z coordinates. The material properties vs. temperature for the ablator and bond are supplied to the computer in tabular form and the computer program automatically determines analytical expressions for the properties by a least square procedure (see Appendix B for details of method). Finally, the grid points which are to be restrained and the external loads which act at grid points are specified.

1. Input Information

The following sequence of punched cards numerically defines the heat shield to be analyzed.

a. FIRST CARD - (72H)

Columns 1 to 72 of this card contains information
to be printed with results

b. SECOND CARD - (6I5, 2F10.2)

Columns 1 - 5 Number of points along the shield - NMAX
6 - 10 Number of points thru the thickness-MMAX
11 - 15 Location of bond line - MB
16 - 20 Number of material property cards - NP

21 - 25 Number of points with radial and axial loads - NL

26 - 30 Number of additional boundary conditions - NB

31 - 40 Surface temperature of ablator

41 - 50 Zero stress temperature

c. THIRD CARD - Properties of Sandwich Core (4F10.2)

Columns 1 - 10 Modulus of elasticity

11 - 20 Poisson's ratio

21 - 30 Coefficient of thermal expansion

31 - 40 Thickness of core

d. FOURTH CARD - Properties of Sandwich Face Plates (4F12.2)

Columns 1 - 10 Modulus of elasticity

11 - 20 Poisson's ratio

21 - 30 Coefficient of thermal expansion

31 - 40 Thickness of single face plate

e. GEOMETRY CARDS - (4F10.2)

One card per point along shield, in order from axis of symmetry to edge (NMAX cards).

Columns 1 - 10 R-ordinate at bond line

11 - 20 Z-ordinate at bond line

21 - 30 Temperature at bond line

31 - 40 Normal thickness of ablator

f. MATERIAL PROPERTY CARDS - (4F10.2)

One card for each temperature (NP cards)

Columns 1 - 10 Temperature

11 - 20 Modulus of elasticity of ablative material

21 - 30 Modulus of elasticity of bond material

31 - 41 Coefficient of thermal expansion for ablator and bond materials

g. LOAD CARDS - (2I5, 2F10.2)

One card for each point which is loaded externally

(NL cards). N and M specify the grid location of the point.

Columns 1 - 5 N (Meridional direction)

6 - 10 M (Thickness direction)

11 - 20 Radial Load

21 - 30 Axial Load } Total load acting on one radian segment

h. BOUNDARY CONDITION CARDS - (3I5)

One card for each point which is restraint (NB cards).

N and M specify the grid location of the point.

Columns 1 - 5 N (Meridional direction)

6 - 10 M (Thickness direction)

11 - 15 Boundary Code

Code = 1 point fixed in R-direction

Code = 2 point fixed in Z-direction

Code = 3 point fixed in both the R and Z-directions

2. Output Information

The following information is generated and printed by the computer program.

- a. Input data
- b. Least squares evaluation of the temperature dependent material property data
- c. Coordinates and temperatures of all grid points
- d. R and Z displacement at all grid points
- e. Average stresses in quadrilateral rings
- f. Stresses in sandwich face plates
- g. Stresses in bond layer

3. Timing

The computer time required by this program for an axisymmetric analysis of a heat shield is approximately

$$\text{time} = A + B \cdot (\text{NMAX}) \cdot (\text{MMAX})^2 \text{ (Seconds)}$$

The constants A and B depend on the specific computer system which is employed. For the IBM 7094 A=20 and B=0.02, and the time required for a 30 x 7 mesh is 50 seconds.

4. Program Listing

A card listing of the FORTRAN II source deck for the automated computer program for the axisymmetric stress analysis of heat shields is given in Appendix E. The program is compiled for a maximum

grid size of 40 points in the meridional direction and 10 points through the thickness. Material properties can be specified by a maximum of 50 cards.

E. EXAMPLES

Several axisymmetric analyses of a heat shield were conducted to evaluate the significance of the various structural parameters. A typical finite element idealization of the heat shield is shown in Figure 3.1.

1. Effect of Mesh Size

The first example was selected to illustrate the effect of mesh size on the accuracy of the displacements and stresses developed within the heat shield. For a structure fixed at the edge, typical results of two analyses with different meshes are shown in Figure 3.2. This example illustrates that two layers of elements in the sandwich shell are adequate for the purposes of predicting stresses. It is of interest to note that the stress distribution varies linearly within the sandwich shell, thereby confirming the assumption made in thin shell theory. The displacements for these two analyses differed by less than one percent.

2. Effect of Ablator Thickness on Stress Distribution

Figure 3.3 shows typical results of three analyses of heat shields with different ablator thicknesses. In general, the magnitude of

- Four Layers in Shell
Four Layers in Ablator
- × Two Layers in Shell
Six Layers in Ablator

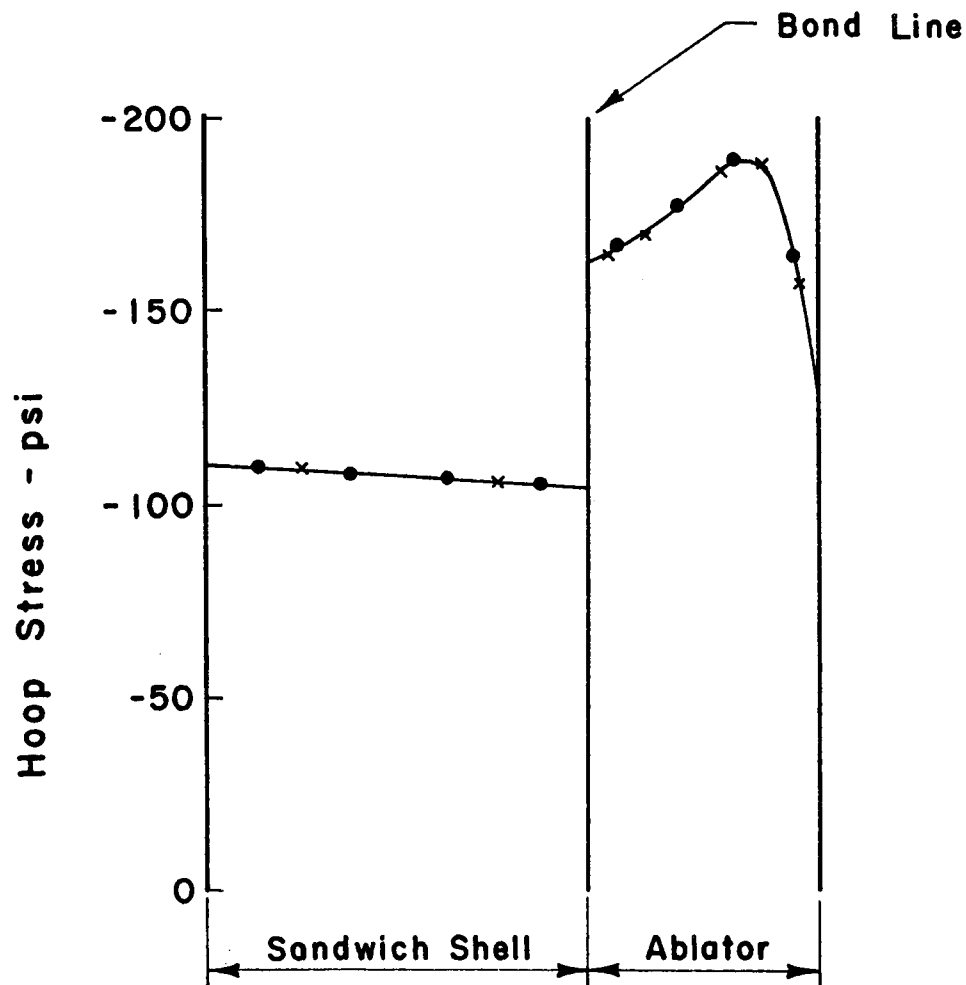


Fig.3.2 Effect of Mesh Size on Stress Distribution

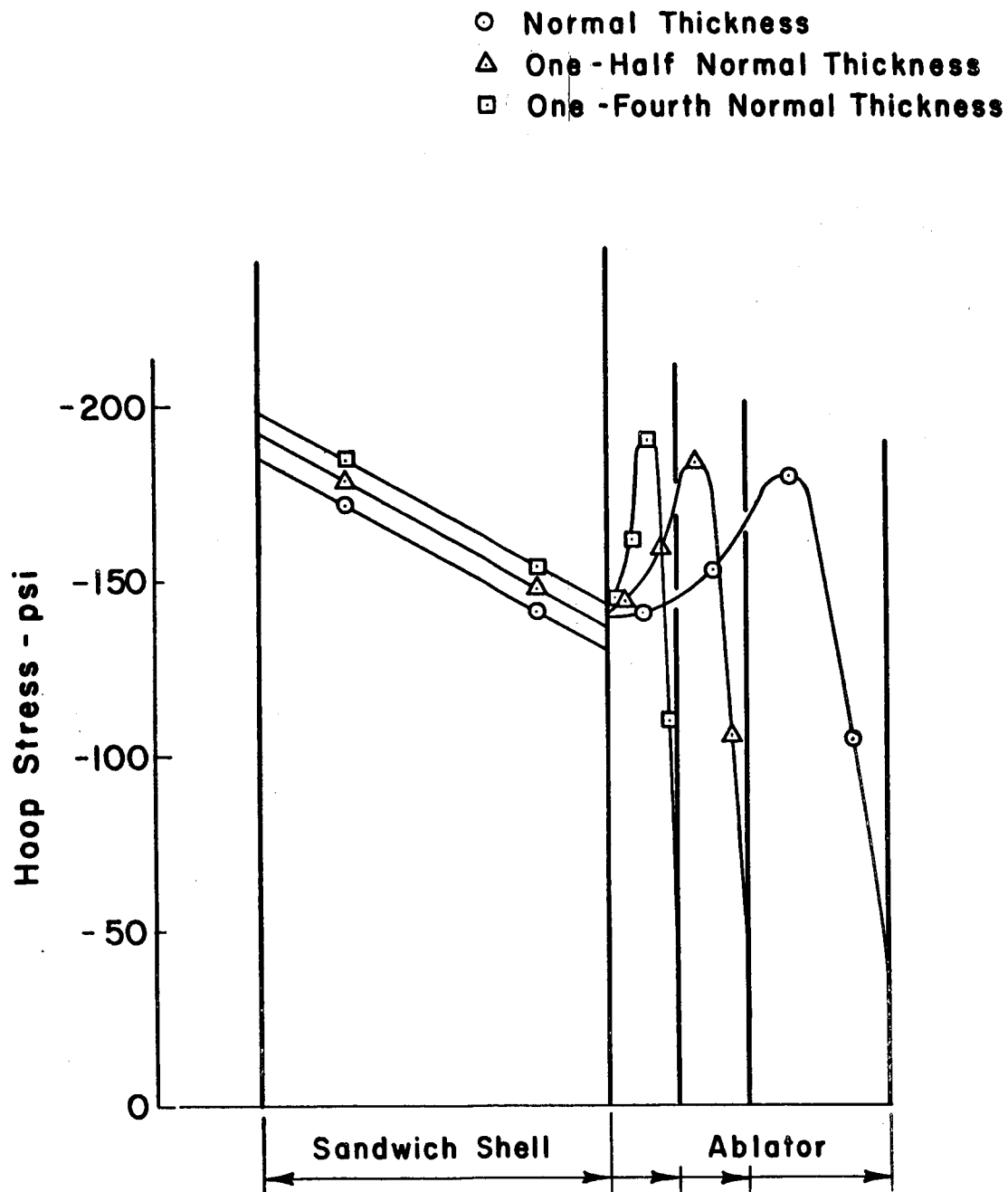


Fig. 3.3 Effect of Thickness of Ablator on Stress Distribution

the maximum stresses in the ablator were in good agreement. This example illustrates that the thickness of the ablator is not an important structural parameter at the temperature of re-entry.

3. Effect of Boundary Conditions on the Behavior of the Heat Shield

The support condition which is imposed on the heat shield is an extremely important parameter. Figure 3.4 illustrates the deflected position of the bond line for two different support conditions. The resulting stresses differ significantly. Therefore, it is important that the boundary condition which is imposed on the finite element system is a realistic approximation of the physical support condition which exists in the actual heat shield.

F. DISCUSSION

The automated computer program presented in this section reduces the analysis of an arbitrary heat shield subjected to axisymmetric thermal or mechanical loads to a simple procedure. The program automatically generates the finite element grid, evaluates temperature-dependent material properties, solves the equilibrium equations for the grid point displacements and calculates stresses within elements, sandwich shell face plates and at the bond layer. Arbitrary boundary conditions can be imposed since any of the grid points may be restrained in either the R or Z directions.

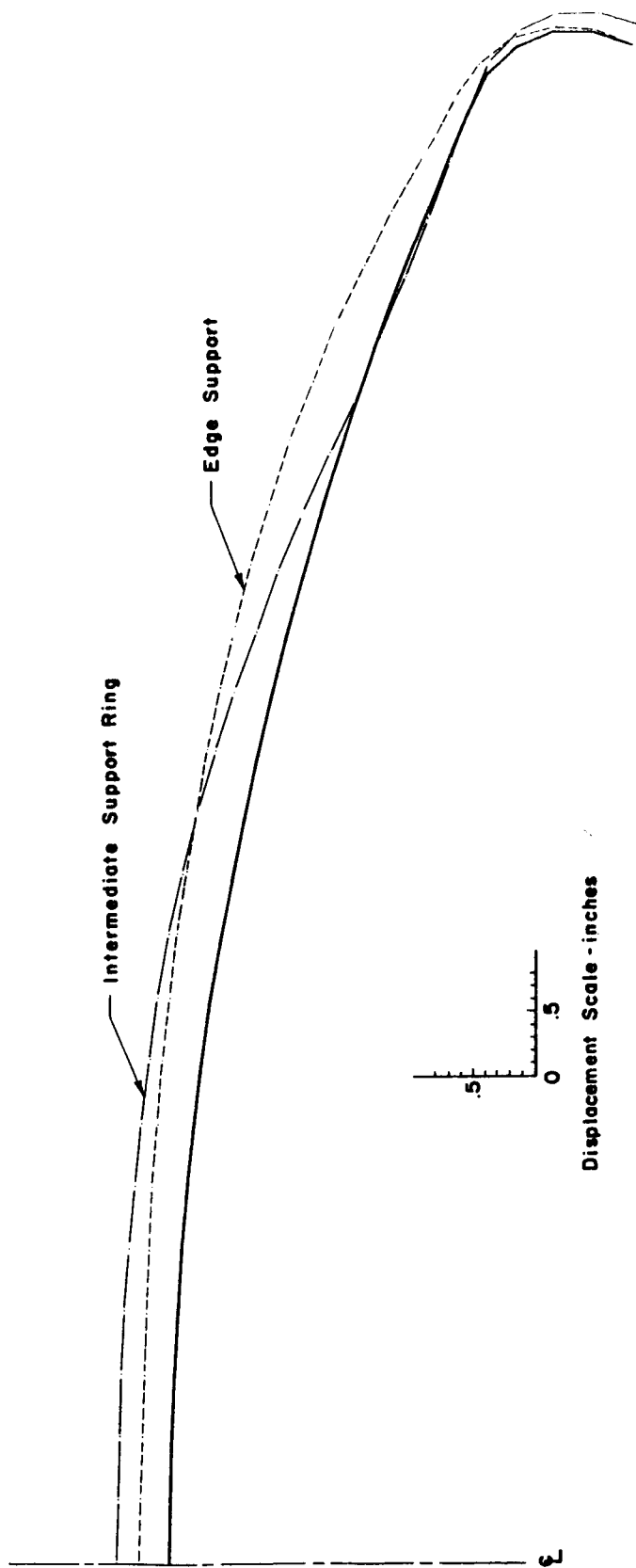


Fig. 3.4 Effect of Boundary Conditions on Deflected Shape (Bond Line)

PART IV: AUTOMATED PROGRAM FOR AXISYMMETRIC HEAT SHIELDS
SUBJECTED TO NON-AXISYMMETRIC LOADS

A. INTRODUCTION

In general, the heat shield of a manned spacecraft is composed of a constant thickness sandwich shell and an ablator which varies in thickness in both the meridional and circumferential directions. The temperature distribution experienced by the heat shield is also non-axisymmetric. Because the ablator, at high temperatures, is not a major structural element, contributing to the overall behavior of the heat shield, an approximation of its properties in the circumferential direction is justified. The approximation, that it is axisymmetric, reduces the stress analysis of a non-axisymmetric heat shield to the stress analysis of an axisymmetric structure subjected to non-axisymmetric thermal loads. This involves the expansion of the temperature distribution and the final displacements of the system in Fourier series. By making use of the orthogonality properties of the harmonic functions the three-dimensional analysis is divided into a series of uncoupled two-dimensional analyses.

B. THEORY FOR THE ANALYSIS OF AN AXISYMMETRIC BODY SUBJECTED
 TO NON-AXISYMMETRIC LOADS

A theory is presented for the analysis of solids of revolution subjected to non-axisymmetric loads which are symmetric about a plane

containing the axis of revolution. Figure 4.1, a view of a plane perpendicular to the axis of revolution, shows the trace of the plane of symmetry. Anisotropic material properties, which are constant along any circumferential line, are included in this formulation.

The structure is idealized as a series of rings with triangular cross-sections; the rings are interconnected at their nodal circles, i.e., at the circles containing the vertices of the triangles, (Figure 4.2). Loads acting on the structure are replaced by statically equivalent concentrated forces acting along the nodal circles.

1. Strain-Displacement Relationship

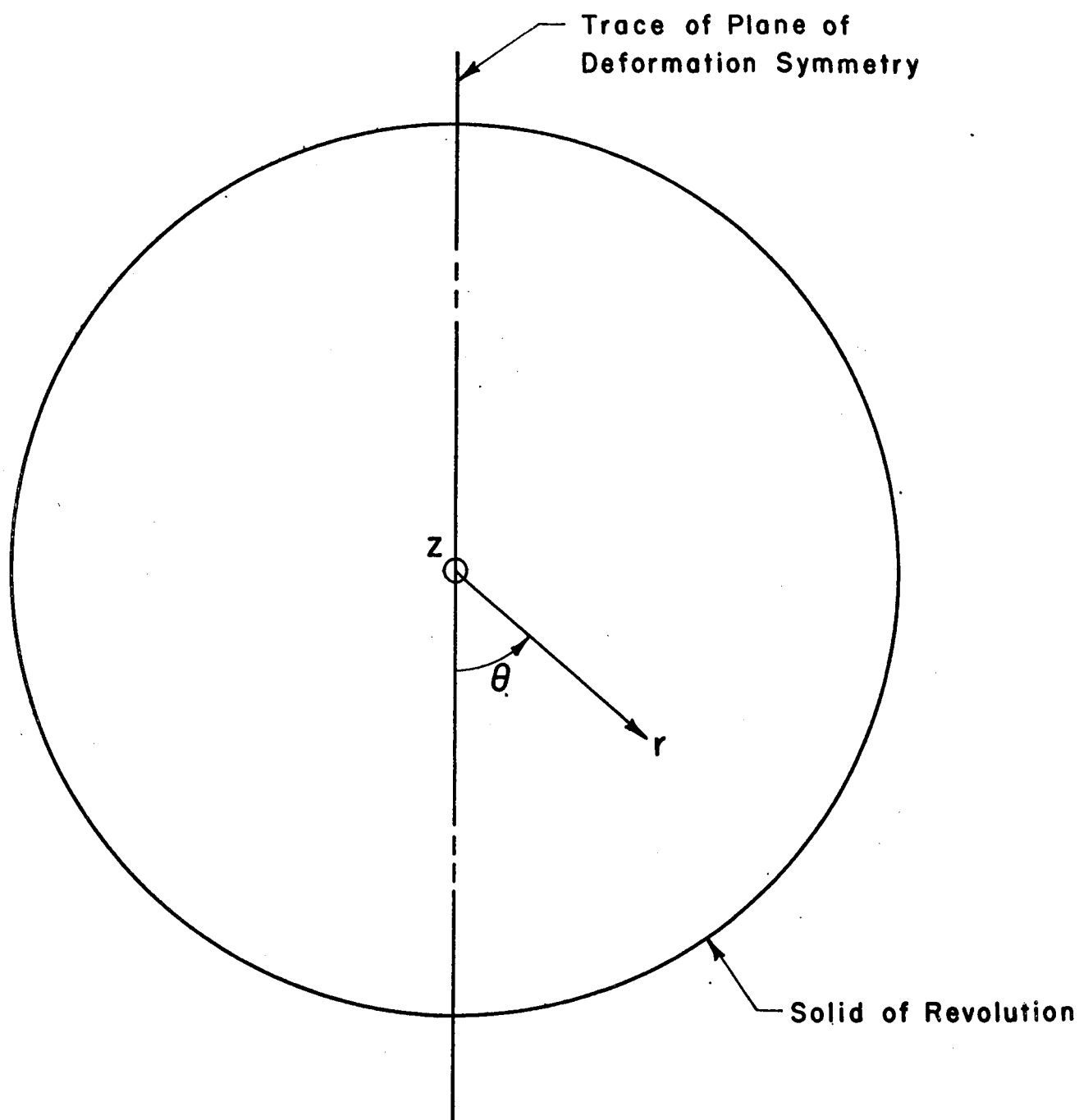
By noting the axisymmetry of the geometry and the material properties of the body and the plane of symmetry for deformations, the displacements in r , θ , z coordinates may be written in the following form:

$$u_r = \sum u_{rn}(r,z) \cos n\theta \quad (4.1a)$$

$$u_\theta = \sum u_{zn}(r,z) \sin n\theta \quad (4.1b)$$

$$u_z = \sum u_{\theta n}(r,z) \cos n\theta \quad (4.1c)$$

Within each ring element the r and z variation of the Fourier coefficients of the displacements are assumed to be linear, i.e.,



**Fig. 4.1 Cylindrical Coordinate System
Embedded in a Solid of Revolution**

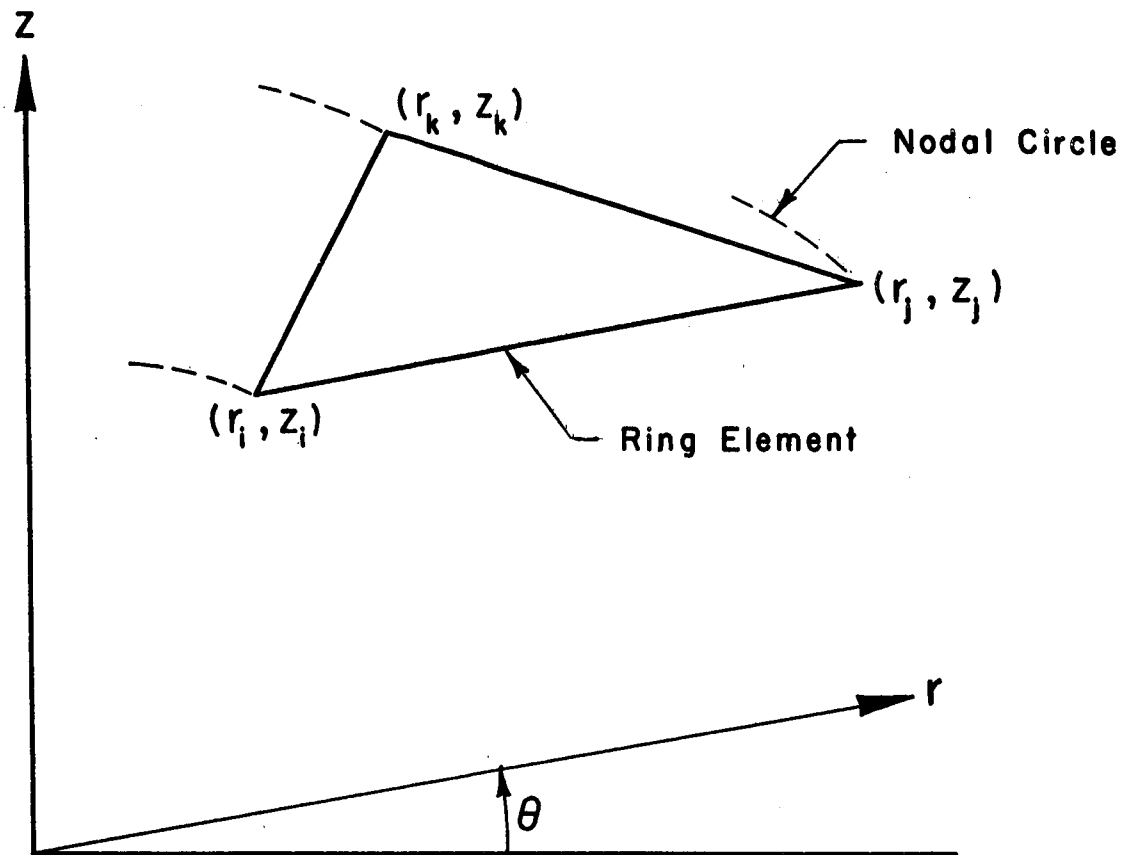


Fig. 4.2 Cross Section of a Ring Element

$$u_{rn} \approx k_{1n} + k_{2n} r + k_{3n} z \quad (4.2a)$$

$$u_{\theta n} \approx k_{4n} + k_{5n} r + k_{6n} z \quad (4.2b)$$

$$u_{zn} \approx k_{7n} + k_{8n} r + k_{9n} z \quad (4.2c)$$

Now expressing the constants k_{mn} in terms of the corner values of the Fourier coefficients of the displacements, i.e., in terms of

$$u_{rn}^i, u_{\theta n}^i, u_{zn}^i, u_{rn}^j, u_{\theta n}^j, u_{zn}^j, u_{rn}^k, u_{\theta n}^k \text{ and } u_{zn}^k$$

$$\begin{bmatrix} k_{1n} & k_{4n} & k_{7n} \\ k_{2n} & k_{5n} & k_{8n} \\ k_{3n} & k_{6n} & k_{9n} \end{bmatrix} = [T] \begin{bmatrix} u_{rn}^i & u_{\theta n}^i & u_{zn}^i \\ u_{rn}^j & u_{\theta n}^j & u_{zn}^j \\ u_{rn}^k & u_{\theta n}^k & u_{zn}^k \end{bmatrix} \quad (4.3a)$$

with

$$[T] = \frac{1}{D} \begin{bmatrix} r_j z_k - z_j r_k & r_k z_i - r_i z_k & r_i z_j - r_j z_i \\ z_j - z_k & z_k - z_i & z_i - z_j \\ r_k - r_j & r_i - r_k & r_j - r_i \end{bmatrix} \quad (4.3b)$$

$$\text{and } D = r_j(z_k - z_i) + r_i(z_j - z_k) + r_k(z_i - z_j) \quad (4.3c)$$

Combining Equation (4.1) with the strain-displacement relationships, the following expressions for the strains are found:

$$\epsilon_r = \frac{\partial u}{\partial r} = \sum \epsilon_{rn} \cos n\theta \quad (4.4a)$$

$$\epsilon_{\theta} = \frac{1}{r} \frac{\partial u_{\theta}}{\partial \theta} + \frac{u_r}{r} = \sum \epsilon_{\theta n} \cos n\theta \quad (4.4b)$$

$$\epsilon_z = \frac{\partial u_z}{\partial z} = \sum \epsilon_{zn} \cos n\theta \quad (4.4c)$$

$$\gamma_{r\theta} = \left(\frac{1}{r} \frac{\partial u_r}{\partial \theta} + \frac{\partial u_{\theta}}{\partial r} - \frac{u_{\theta}}{r} \right) = \sum \gamma_{r\theta n} \sin n\theta \quad (4.4d)$$

$$\gamma_{rz} = \left(\frac{\partial u_z}{\partial r} + \frac{\partial u_r}{\partial z} \right) = \sum \gamma_{rzn} \cos n\theta \quad (4.4e)$$

$$\gamma_{\theta z} = \left(\frac{\partial u_{\theta}}{\partial z} + \frac{1}{r} \frac{\partial u_z}{\partial \theta} \right) = \sum \gamma_{\theta zn} \sin n\theta \quad (4.4f)$$

where

$$\epsilon_{rn} = \frac{\partial u_{rn}}{\partial r} \quad (4.5a)$$

$$\epsilon_{\theta n} = \frac{1}{r} (n u_{\theta n} + u_r) \quad (4.5b)$$

$$\epsilon_{zn} = \frac{\partial u_{zn}}{\partial z} \quad (4.5c)$$

$$\gamma_{r\theta n} = \frac{\partial u_{\theta n}}{\partial r} - \frac{u_{\theta n}}{r} - \frac{n u_{rn}}{r} \quad (4.5d)$$

$$\gamma_{rzn} = \frac{\partial u_{zn}}{\partial r} + \frac{\partial u_{rn}}{\partial z} \quad (4.5e)$$

$$\gamma_{\theta zn} = \frac{\partial u_{\theta n}}{\partial z} - n \frac{u_{zn}}{r} \quad (4.5f)$$

Within a given ring the approximate values of strain for the harmonic n are calculated by combining Equations (4.2), (4.3) and (4.5). The hoop strain is assumed to be constant within the ring and $\frac{u_{rn}}{r}$ is approximated by

$$\frac{u_{rn}^i}{3\bar{r}} + \frac{u_{rn}^j}{3\bar{r}} + \frac{u_{rn}^k}{3\bar{r}}, \text{ with } \bar{r} = \frac{1}{3}(r_i + r_j + r_k)$$

Thus, for the harmonic n the six components of strain within the element are given in terms of the nine corner displacements by the following matrix equation:

$$[\epsilon_n] = [G_n][u_n] \quad (4.6a)$$

The strain-displacement transformation matrix (for convenience it is written in its transposed form) is defined on the next page, Eq. (4.6b).

2. Stress-Strain Relationship

The stress-strain relationship for the harmonic n is written in the following symbolic form:

$$[G_n] = [C][\epsilon_n] + [\tau_n] \quad (4.7a)$$

For an isotropic material this becomes

$$\begin{bmatrix} \sigma_{rn} \\ \sigma_{\theta n} \\ \sigma_{zn} \\ \sigma_{r\theta n} \\ \sigma_{rzn} \\ \sigma_{\theta zn} \end{bmatrix} = \begin{bmatrix} \bar{\alpha} & \bar{\beta} & \bar{\beta} & 0 & 0 & 0 \\ \bar{\beta} & \bar{\alpha} & \bar{\beta} & 0 & 0 & 0 \\ \bar{\beta} & \bar{\beta} & \bar{\alpha} & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix} \begin{bmatrix} \epsilon_{rn} \\ \epsilon_{\theta n} \\ \epsilon_{zn} \\ \epsilon_{r\theta n} \\ \epsilon_{rzn} \\ \epsilon_{\theta zn} \end{bmatrix} + \begin{bmatrix} \tau_n \\ \tau_n \\ \tau_n \\ 0 \\ 0 \\ 0 \end{bmatrix} \quad (4.7b)$$

$$\begin{aligned}
 [G_n]^T = & \begin{bmatrix}
 \frac{z_j - z_k}{D} & \frac{1}{3\bar{r}} & 0 & -\frac{n}{3\bar{r}} & \frac{r_k - r_j}{D} & 0 \\
 \frac{z_k - z_i}{D} & \frac{1}{3\bar{r}} & 0 & -\frac{n}{3\bar{r}} & \frac{r_i - r_k}{D} & 0 \\
 \frac{z_i - z_j}{D} & \frac{1}{3\bar{r}} & 0 & -\frac{n}{3\bar{r}} & \frac{r_j - r_i}{D} & 0 \\
 0 & \frac{1}{3\bar{r}} & 0 & (\frac{z_j - z_k}{D} - \frac{1}{3\bar{r}}) & 0 & \frac{r_k - r_j}{D} \\
 0 & \frac{n}{3\bar{r}} & 0 & (\frac{z_j - z_k}{D} - \frac{1}{3\bar{r}}) & 0 & \frac{r_k - r_j}{D} \\
 0 & \frac{n}{3\bar{r}} & 0 & (\frac{z_k - z_i}{D} - \frac{1}{3\bar{r}}) & 0 & \frac{r_i - r_k}{D} \\
 0 & 0 & \frac{r_k - r_j}{D} & 0 & \frac{z_j - z_k}{D} & -\frac{n}{3\bar{r}} \\
 0 & 0 & \frac{r_i - r_k}{D} & 0 & \frac{z_k - z_i}{D} & -\frac{n}{3\bar{r}} \\
 0 & 0 & \frac{r_j - r_i}{D} & 0 & \frac{z_i - z_j}{D} & -\frac{n}{3\bar{r}}
 \end{bmatrix} \quad (4.6b)
 \end{aligned}$$

where
$$\bar{\alpha} = \frac{(1-\nu) E}{(1+\nu)(1-2\nu)} \quad (4.8a)$$

$$\bar{\beta} = \frac{\nu E}{(1+\nu)(1-2\nu)} \quad (4.8b)$$

$$\mu = \frac{E}{2(1+\nu)} \quad (4.8c)$$

$$\tau_n = - \frac{E\alpha}{(1-2\nu)} T_n \quad (4.8d)$$

T_n is the Fourier coefficient for the expansion of the average temperature change within the ring

$$T = \sum T_n \cos n\theta \quad (4.9)$$

3. Equilibrium Equation for Harmonic n

By recognizing the orthogonality of the harmonic functions the same procedure which was used in Part I may be used to develop the equilibrium equations for an element subjected to harmonic loading.

Therefore, Equation (1.15) is rewritten as

$$[S_n] = [k_n] [u_n] + [L_n] \quad (4.10)$$

where
$$[k_n] = \int [G_n]^T [C] [G_n] dV \quad (4.11)$$

$$[L_n] = \int [G_n]^T [\tau_n] dV \quad (4.12)$$

Within a ring the matrices $[G_n]$ and $[C]$ are not a function of space; therefore, Equations (4.11) and (4.12) reduce to

$$[k_n] = V \cdot [G_n]^T [C] [G_n] \quad (4.13)$$

$$[L_n] = V \cdot [G_n]^T [\tau_n] \quad (4.14)$$

where the volume V is given by Equation (2.10). Equilibrium of the overall structure requires that the sum of the nodal circle forces for all rings with a common nodal circle must equal the applied nodal force. This results on an equation of the following form for each harmonic:

$$[K_n] [r_n] = [R_n] \quad (4.15)$$

where the displacement vector $[r_n]$ contains all the displacement amplitudes u_{rn} , $u_{\theta n}$ and u_{zn} for all nodal circles in the system.

The equilibrium of the face plates is incorporated by a similar procedure. Appendix C gives the details of this development.

4. Determination of Displacements and Stresses

The number of harmonics required to represent the three-dimensional temperature distribution indicates the number of two-dimensional problems which must be solved. For each harmonic Equation (4.15) must be solved for the unknown displacement amplitudes. The corresponding strain amplitudes are calculated for each finite element by Equation (4.6) and then stress amplitudes are found by the application of Equation (4.7). The final displacements of the system for any angle are calculated from Equation (4.1). The final stresses are determined from the stress amplitudes by the following equations:

$$\sigma_r(r, z, \theta) = \sum \sigma_{rn} \cos n\theta \quad (4.16a)$$

$$\sigma_z(r, z, \theta) = \sum \sigma_{zn} \cos n\theta \quad (4.16b)$$

$$\sigma_\theta(r, z, \theta) = \sum \sigma_{\theta n} \sin n\theta \quad (4.16c)$$

$$\sigma_{rz}(r, z, \theta) = \sum \sigma_{rzn} \cos n\theta \quad (4.16d)$$

$$\sigma_{r\theta}(r, z, \theta) = \sum \sigma_{r\theta n} \sin n\theta \quad (4.16e)$$

$$\sigma_{\theta z}(r, z, \theta) = \sum \sigma_{\theta zn} \sin n\theta \quad (4.16f)$$

C. COMPUTER PROGRAM

The use of the non-axisymmetric heat shield program is similar to the axisymmetric program (Part III). The only additional input required is the three-dimensional temperature distribution. The computer program automatically develops the necessary Fourier coefficients for the temperature distribution and sums the series of two-dimensional analyses to produce the final displacements and stresses in the system.

1. Input Information

The following sequence of punched cards numerically defines the heat shield to be analyzed:

a. FIRST CARD - (72H)

Columns 1 to 72 of this card contains information to be pointed with results

b. SECOND CARD - (615, 2F10.2)

Columns 1 - 5 Number of points along meridian
of shield - NMAX
6 - 10 Number of points through thickness -
MMAX
11 - 15 Location of bond line - MB
16 - 20 Number of material property cards - NP
21 - 25 Number of harmonic to be used in
analysis - NL
26 - 30 Number of boundary condition cards - NB
31 - 40 Surface temperature of ablator
41 - 50 Temperature of zero stress

c. THIRD CARD - Properties of Sandwich Core (4F10.2)

Columns 1 - 10 Modulus of elasticity
11 - 20 Poisson's ratio
21 - 30 Coefficient of thermal expansion
31 - 40 Thickness of core

d. FOURTH CARD - Properties of Sandwich Face Plates (4F10.2)

Columns 1 - 10 Modulus of elasticity
11 - 20 Poisson's ratio
21 - 30 Coefficient of thermal expansion
31 - 40 Thickness of single face plate

e. GEOMETRY CARDS - (4F10.2)

One card per point along shield in order from axis of
symmetry to edge (NMAX cards).

Columns 1 - 10 R-ordinate at bond line
11 - 20 Z-ordinate at bond line
21 - 30 Temperature at bond line
31 - 40 Normal thickness of ablator

The temperature information is used by the program to determine the axisymmetric temperature-dependent material properties.

f. MATERIAL PROPERTY CARDS - (4F10.2)

One card for each temperature (NP cards)

Columns 1 - 10 Temperature
11 - 20 Modulus of elasticity of ablative material
21 - 30 Modulus of elasticity of bond material
31 - 41 Coefficient of thermal expansion for ablative and bond materials

g. THREE-DIMENSIONAL TEMPERATURE DISTRIBUTION CARDS

A table of bond line temperature values at 10 degree increments along 9 circumferential lines is punched in the following form:

1st. card - (9F8.0)

R-ordinates of 9 circumferential points on bond line

2nd card - (9F8.0)

Z-ordinates of 9 circumferential points on bond line

3rd to 21st card - (9F8.0)

One card for each 10 degree increment (0 to 180°).

Each card contains the 9 temperatures which correspond to the above R and Z-ordinates.

h. BOUNDARY CONDITION CARDS - (3I5)

One card per restrained nodal circle (NB cards)

Column 1 - 5 N } mesh point N, M
6 - 10 M }

11 - 15 = 1 restrained in R-direction
2 restrained in θ -direction
3 restrained in Z-direction
4 restrained in R and θ -directions
5 restrained in R and Z-directions
6 restrained in θ and Z-directions
7 restrained in R, θ and Z directions

i. PRINT ANGLE CARDS - (1F5.0)

Column 1 - 5 angle θ

For each "print angle card" a complete set of displacement and stresses are printed for angle θ .

2. Output Information

The following information is generated and printed by the computer program:

- a. Input data
- b. Least squares evaluation of the temperature-dependent material property data
- c. Coordinates and temperature of all grid points
- d. Two-dimensional Fourier temperature coefficients
- e. For each print angle
 - (1) R, Z, and θ displacement at all grid points
 - (2) Average stresses in quadrilateral rings
 - (3) Stresses in sandwich face plates

3. Timing

The computer time required by this program for the non-axisymmetric analysis of a heat shield is approximately

$$\text{time} = A + B \cdot (NMAX) \cdot (MMAX)^2 \cdot NL \text{ (seconds)}$$

For the IBM 7094 computer $A=20$ and $B=0.05$, and the time required for a 30×7 mesh with 4 harmonics is approximately 5 minutes.

4. Program Listing

A card listing of the FORTRAN II source deck for the computer program for the non-axisymmetric analysis of heat shields is given in Appendix E. The program is compiled for a maximum grid size of 30 points in the meridional direction and 8 points through the thickness. A maximum of 10 harmonics may be considered. Material properties can be specified by a maximum of 50 cards.

Standard input tape 5 and output tape 6 are used by the program. Tape 20 is used for temporary storage within the program; it may be necessary to change this tape unit to conform with local computer center policy.

D. EXAMPLES

Two analyses of axisymmetric heat shields subjected to non-axisymmetric temperature distribution were conducted to illustrate the application of the

program. In both cases, the axisymmetric finite element mesh was similar to the mesh given by Figure 3.1. For the purpose of reference the station layout along the bond line is shown in Figure 4.3. The three-dimensional bond layer temperature and ablator thickness distribution for angles $\theta = 0, 90^\circ, 180^\circ$ are plotted in Figure 4.4.

In Analysis A the axisymmetric properties of the ablator are assumed to be equal to the properties of the actual ablator at $\theta = 0$. In Analysis B the ablator properties at $\theta = 180^\circ$ are used. For both analyses the surface temperature of the ablator is 1000 F and the temperature at the bond surface is given by Figure 4.5. Station 20 (Figure 4.3) is restrained at the inside surface of the shield to simulate the effect of an intermediate support ring.

The computer output for a non-axisymmetric analysis contains displacements and stresses at many points in the heat shield; however, only the typical results are presented. For Analysis A the deflected shape of the bond line at three sections is plotted in Figure 4.6a. The non-axisymmetric behavior is significant. The displacements from Analysis B are shown in Figure 4.6b; they are essentially the same as those found by Analysis A. This again indicates that the ablator's thickness and property variations in the circumferential direction are not of major importance at these temperatures. Hoop stresses at station 15 are

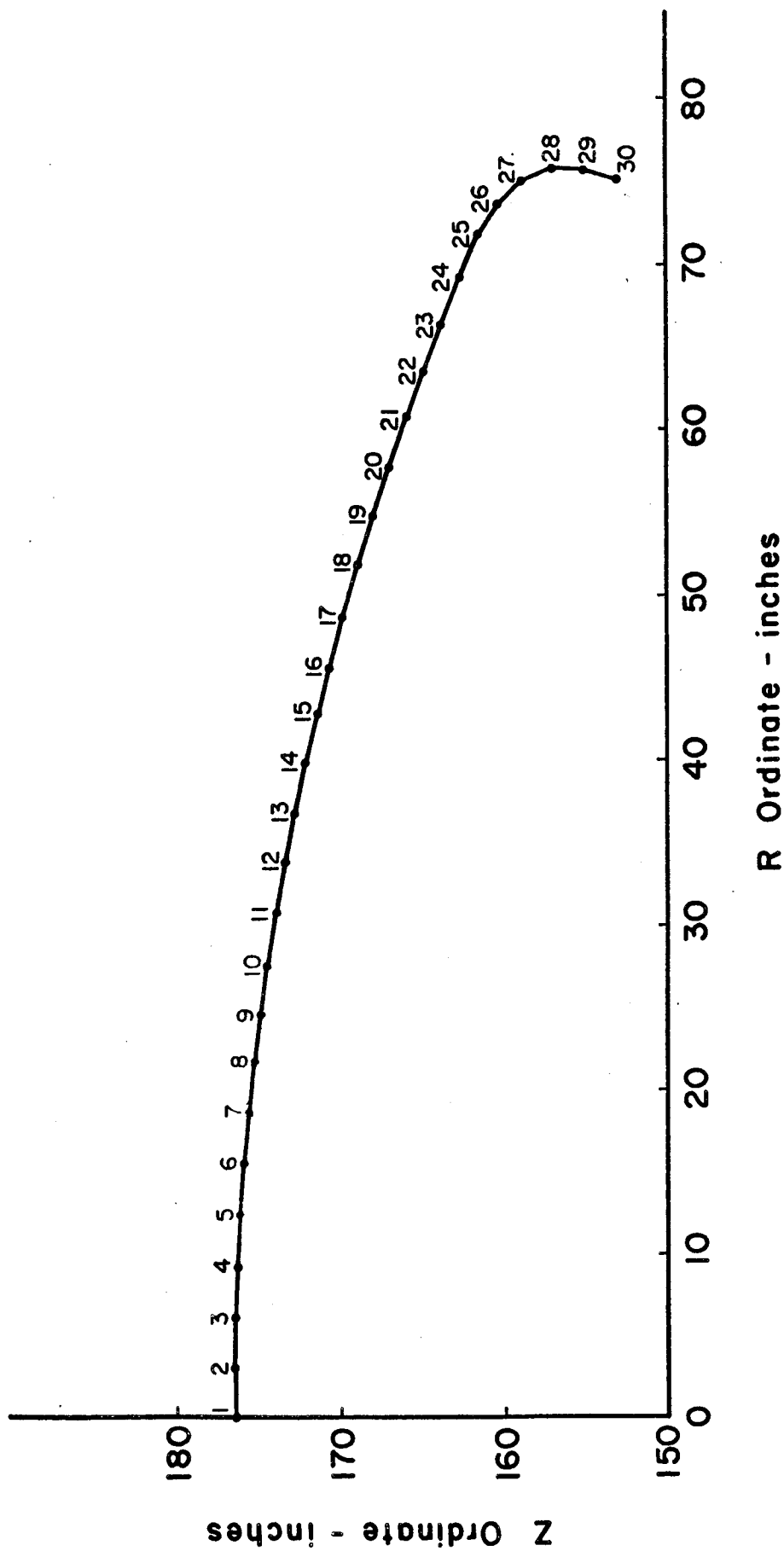


Fig. 4.3 Station along Bond Line

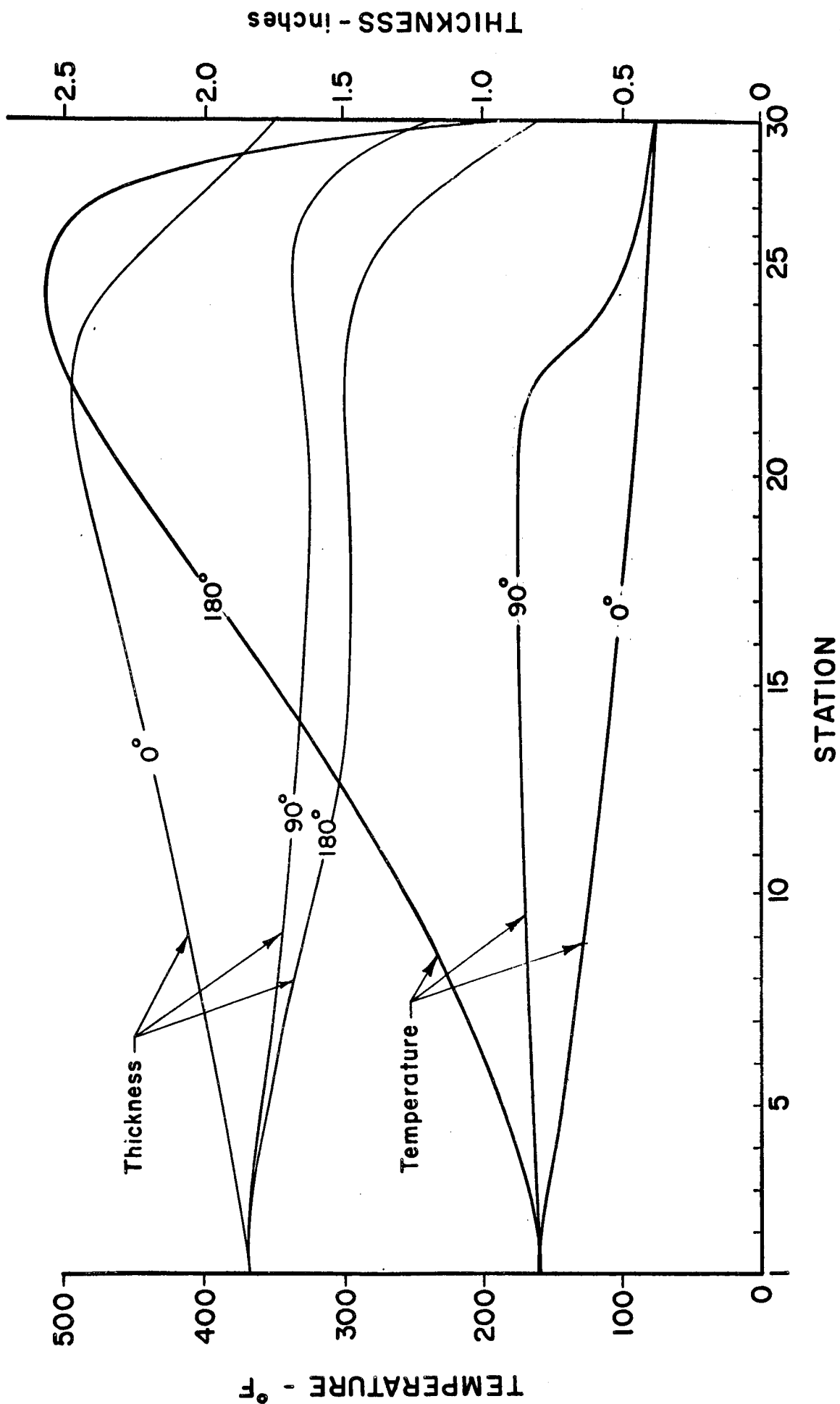


Fig. 4.4 Bond Layer Temperature and Ablator Thickness Distribution

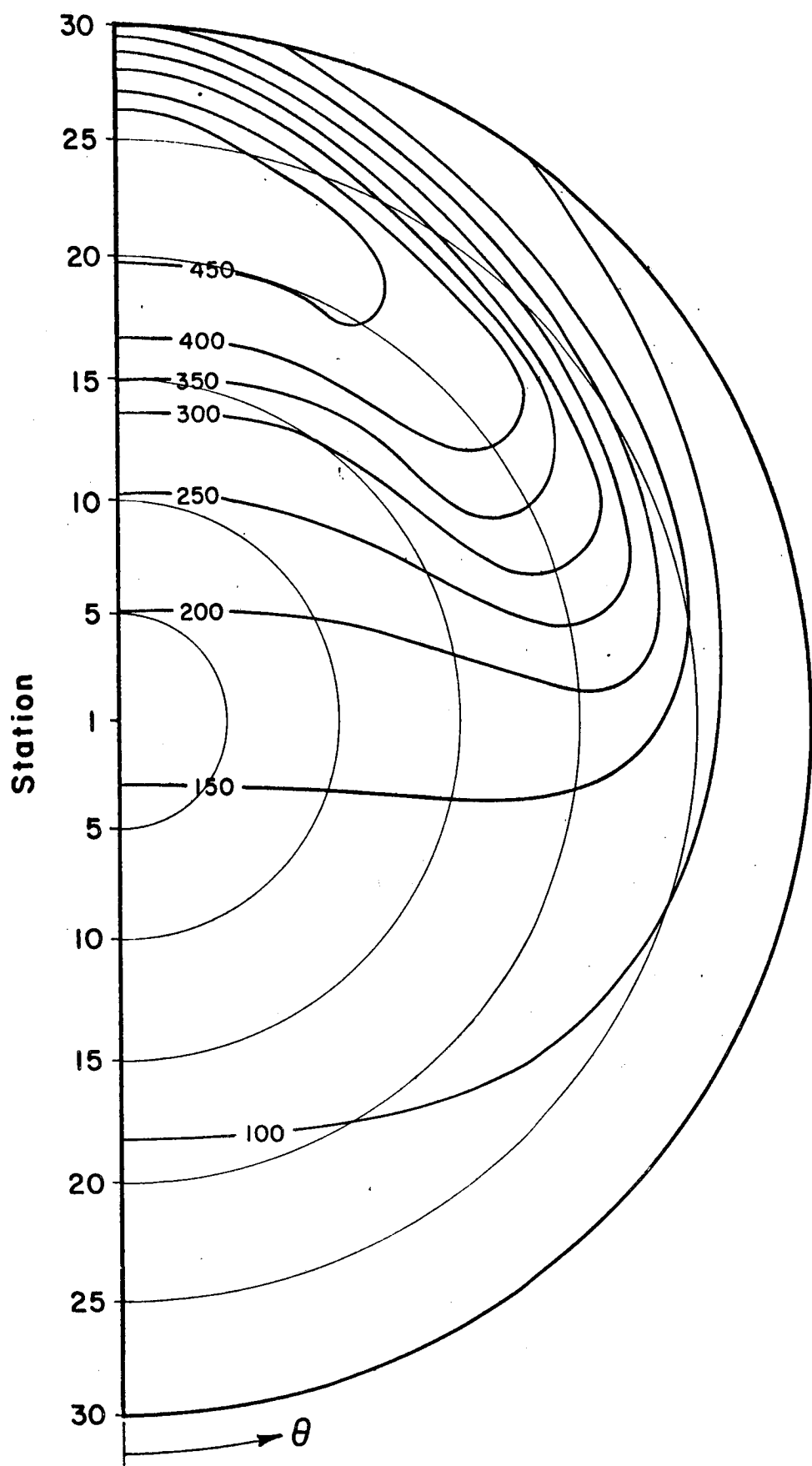


Fig. 4.5 Temperature at Bond Surface

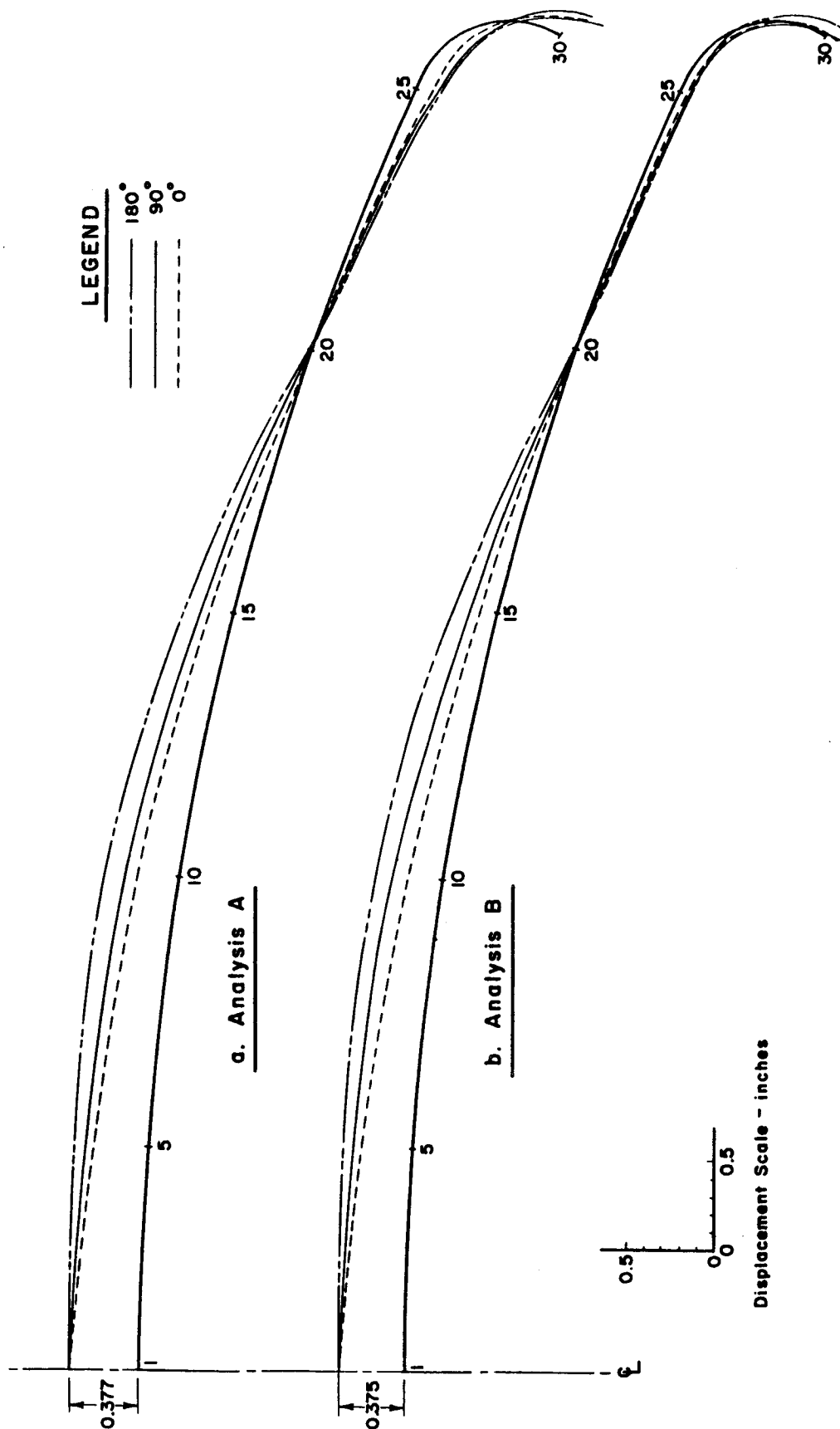


Fig. 4.6 Deflected Shape at Bond Surface

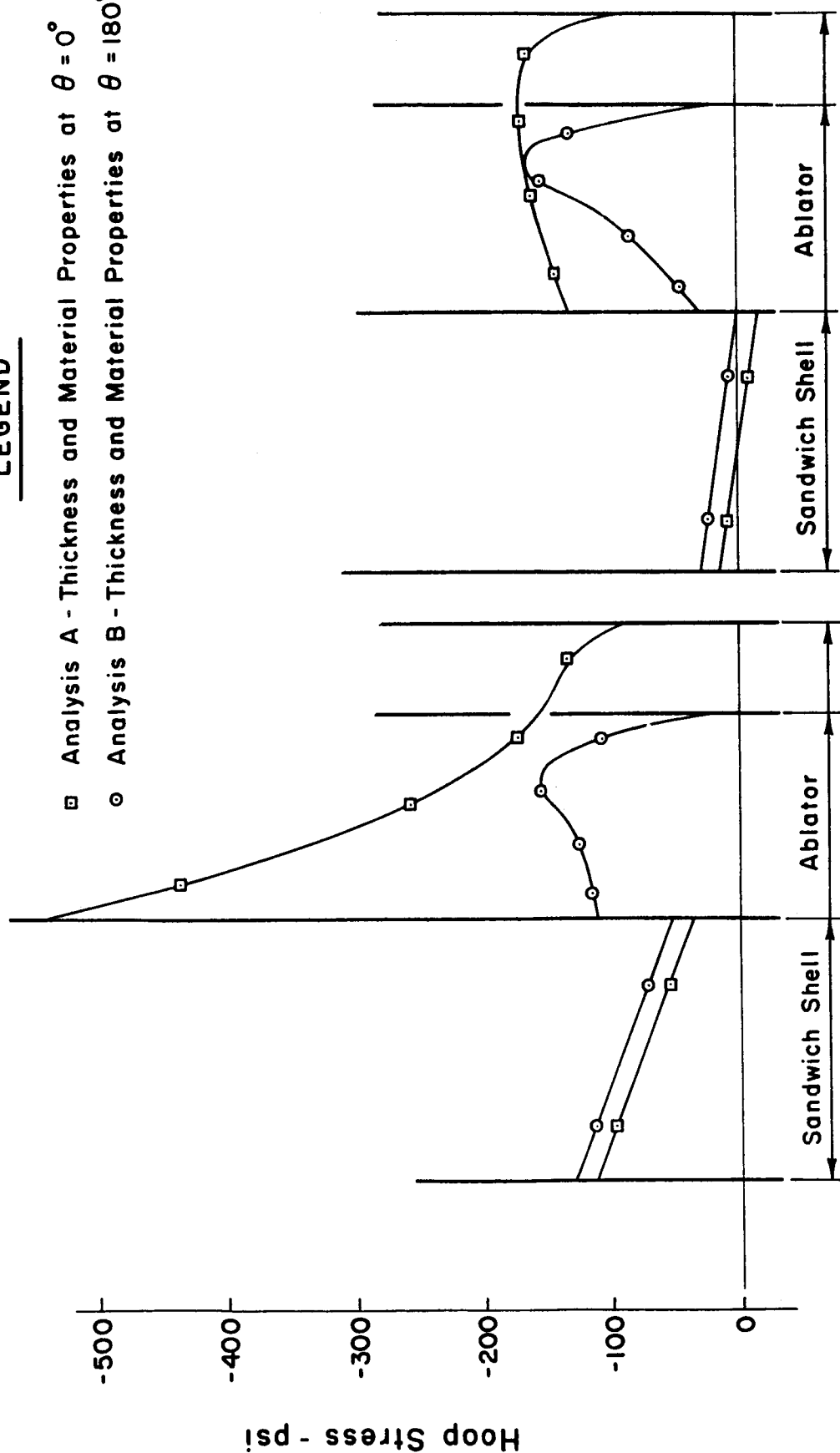
plotted in Figure 4.7 for two values of θ . In both analyses the stresses within the sandwich shell are in fair agreement since the material properties do not change at these temperatures. However, within the ablator, where the material properties are strongly temperature-dependent, the stresses differ significantly. Of course, the particular solution is most reasonable if the assumed material properties correspond with those at the position of the desired stress. Hence, for $\theta = 0$, Analysis A is considered the best approximation and similarly for $\theta = 180^\circ$, Analysis B is the most reasonable.

E. DISCUSSION

In this section a method and the resulting computer program are presented for the analysis of axisymmetric heat shields subjected to non-axisymmetric thermal loads. The program may be used to analyze an approximate solution to a non-axisymmetric heat shield if a number of solutions are obtained and then are judiciously evaluated. At the section where the assumed axisymmetric ablator properties (temperature and thickness) correspond to the local properties, the stresses will be a good approximation of the actual state of stress in the non-axisymmetric heat shield. Therefore, for each angle θ for which stresses are desired a separate structure must be evaluated.

LEGEND

- Analysis A - Thickness and Material Properties at $\theta = 0^\circ$
- Analysis B - Thickness and Material Properties at $\theta = 180^\circ$



a. Hoop Stress - Station at $\theta = 180^\circ$

b. Hoop Stress - Station at $\theta = 0^\circ$

Fig. 4.7 Typical Results of Non-Axisymmetric Heat Shield Analysis

It should be pointed out that the computer program can be extended to include non-axisymmetric pressure loading, displacement boundary conditions and the effects of elastic supports. However, this additional investigation was beyond the scope of the present effort.

APPENDIX A

SOLUTION OF EQUILIBRIUM EQUATIONS

The equilibrium equations for a system of finite elements may be written in the following form:

$$A_{11} X_1 + A_{12} X_2 + A_{13} X_3 \text{ ----- } + A_{1N} X_N = B_1 \quad (\text{Ala})$$

$$A_{21} X_1 + A_{22} X_2 + A_{23} X_3 \text{ ----- } + A_{2N} X_N = B_2 \quad (\text{Alb})$$

$$A_{31} X_1 + A_{32} X_2 + A_{33} X_3 \text{ ----- } + A_{3N} X_N = B_3 \quad (\text{Alc})$$

$$A_{N1} X_1 + A_{N2} X_2 + A_{N3} X_3 \text{ ----- } + A_{NN} X_N = B_N \quad (-)$$

or symbolically

$$[A] [X] = [B] \quad (\text{Al})$$

where

$$[A] = \text{the stiffness matrix}$$

$$[X] = \text{the unknown displacements}$$

$$[B] = \text{the applied loads}$$

Gaussian Elimination

The first step in the solution of the above set of equations is to solve Equation (A1a) for X_1 , Or

$$X_1 = B_1/A_{11} - (A_{12}/A_{11}) X_2 - (A_{13}/A_{11}) X_3 - \dots - (A_{1N}/A_{11}) X_N \quad (A2)$$

If Equation (A2) is substituted into Equations (A1b, c, ..., N) a modified set of N-1 equations is determined.

$$A_{22}^1 X_2 + A_{23}^1 X_3 - \dots + A_{2N}^1 X_N = B_2^1 \quad (A3a)$$

$$A_{32}^1 X_2 + A_{33}^1 X_3 - \dots + A_{3N}^1 X_N = B_3^1 \quad (A3b)$$

$$A_{N2}^1 X_2 + A_{N3}^1 X_3 - \dots + A_{NN}^1 X_N = B_N^1$$

where $A_{ij}^1 = A_{ij} - A_{i1} A_{1j}/A_{11} \quad i, j = 2, \dots, N \quad (A4a)$

$$B_i^1 = B_i - A_{i1} B_1/A_{11} \quad i = 2, \dots, N \quad (A4b)$$

A similar procedure is used to eliminate X_2 from Equation (A3), etc.

A general algorithm for the elimination of X_n may be written as

$$x_n = (B_n^{n-1}/A_{nn}^{n-1}) - \sum (A_{nj}^{n-1}/A_{nn}^{n-1}) x_j \quad j = n+1, \dots, N \quad (A5)$$

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} (A_{nj}^{n-1}/A_{nn}^{n-1}) \quad i, j = n+1, \dots, N \quad (A6)$$

$$B_i^n = B_i^{n-1} - A_{in}^{n-1} (B_n^{n-1}/A_{nn}^{n-1}) \quad i = n+1, \dots, N \quad (A7)$$

Equations A5, A6 and A7 may be rewritten in compact form:

$$x_n = D_n - \sum C_{nj} x_j \quad j = n+1, \dots, N \quad (A8)$$

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} C_{nj} \quad i, j = n+1, \dots, N \quad (A9)$$

$$B_i^n = B_i^{n-1} - A_{in}^{n-1} D_n \quad i = n+1, \dots, N \quad (A10)$$

where $D_n = B_n^{n-1}/A_{nn}^{n-1}$

$$C_{nj} = A_{nj}^{n-1}/A_{nn}^{n-1}$$

After the above procedure is applied $N-1$ times the original set of equations is reduced to the following single equation

$$A_{NN}^{N-1} x_N = B_N^{N-1}$$

which is solved directly for X_N

$$X_N = B_N^{N-1} / A_{NN}^{N-1}$$

In terms of the previous notation, this is

$$X_N = D_N \quad (A11)$$

The remaining unknowns are determined in reverse order by the repeated application of Equation (A8).

Simplification for Band Matrices

For many finite element systems it is possible to place the stiffness matrix in a "band" form which results in the concentration of the elements of the stiffness matrix along the main diagonal. Therefore, the following simplifications in the general algorithm (Equations A8, A9 and A10) are possible:

$$X_n = D_n - \sum C_{nj} X_j \quad j = n+1, \dots, n+M-1 \quad (A12)$$

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} C_{nj} \quad i, j = n+1, \dots, n+M-1 \quad (A13)$$

$$B_i^n = B_i^{n-1} - A_{in}^{n-1} D_n \quad i = n+1, \dots, n+M-1 \quad (A14)$$

where M is the band width of the matrix.

The number of numerical operations can further be reduced by recognizing that the reduced matrix at any stage of procedure is symmetric. Accordingly, Equation (A13) may be replaced by the following equation:

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} C_{nj} \quad \begin{array}{l} i = n+1, \dots, n+M-1 \\ j = i, \dots, n+M-1 \end{array} \quad (A15)$$

since

$$A_{ji}^n = A_{ij}^n$$

The number of numerical operations required for the solution of a band matrix is proportional to NM^2 as compared to N^3 which is required for the solution of a full matrix. Also, the computer storage required by the band matrix procedure is NM as compared to N^2 required by a set of N arbitrary equations.

This technique has been used in the automated axisymmetric program for the analysis of a typical heat shield idealized by a 10×40 mesh of quadrilateral elements. A solution to 800 simultaneous equations was necessary, which required less than two minutes of computing time on the IBM 7094.

APPENDIX B

MATRIX FORMULATION OF THE LEAST SQUARE CURVE-FIT PROCEDURE

Consider the problem of selecting the "best" polynomial of the form $y = C_1 + C_2X + C_3X^2 + C_4X^3, \dots, C_NX^{N-1}$ to represent the following set of data points:

$$X_1, Y_1; X_2, Y_2; \text{-----}; X_M, Y_M$$

If the above polynomial is evaluated at points X_1 to X_M , M equations of the following form are found:

$$C_1 + C_2 X_m + C_3 X_m^2 + \text{-----} C_N X_m^{N-1} = Y_m \quad m = 1, \dots, M$$

or in matrix form

$$\begin{bmatrix} 1 & X_1 & X_1^2 & \text{-----} & X_1^{N-1} \\ 1 & X_2 & X_2^2 & \text{-----} & X_2^{N-1} \\ \text{-----} & \text{-----} & \text{-----} & \text{-----} & \text{-----} \\ 1 & X_m & X_m^2 & \text{-----} & X_m^{N-1} \\ \text{-----} & \text{-----} & \text{-----} & \text{-----} & \text{-----} \\ 1 & X_M & X_M^2 & \text{-----} & X_M^{N-1} \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ - \\ - \\ C_N \end{bmatrix} = \begin{bmatrix} Y_1 \\ Y_2 \\ - \\ Y_m \\ - \\ Y_M \end{bmatrix} \quad (\text{Bla})$$

or symbolically

$$[A][C] = [Y] \quad (B1b)$$

where

$$[A] = \text{a } M \times N \text{ matrix}$$

$$[C] = \text{a } N \times 1 \text{ matrix}$$

$$[Y] = \text{a } M \times 1 \text{ matrix}$$

If Equation (B1) is premultiplied by $[A]^T$, a set of N linear equations in N unknowns is created. Consequently,

$$[B][C] = [D] \quad (B2)$$

where

$$[B] = [A]^T [A] = \text{a } N \times N \text{ matrix}$$

$$[D] = [A]^T [Y] = \text{a } N \times 1 \text{ matrix}$$

Equation (B2) can now be solved directly for the unknown coefficients $[C]$.

This procedure is numerically equivalent to the standard least square procedure. However, it is presented here in a form which is readily programmed for the digital computer. The technique is not restricted to polynomials.

Figure B1 illustrates the application of the method in the evaluation of the elastic modulus for temperature dependent materials. A fourth order polynomial was used.

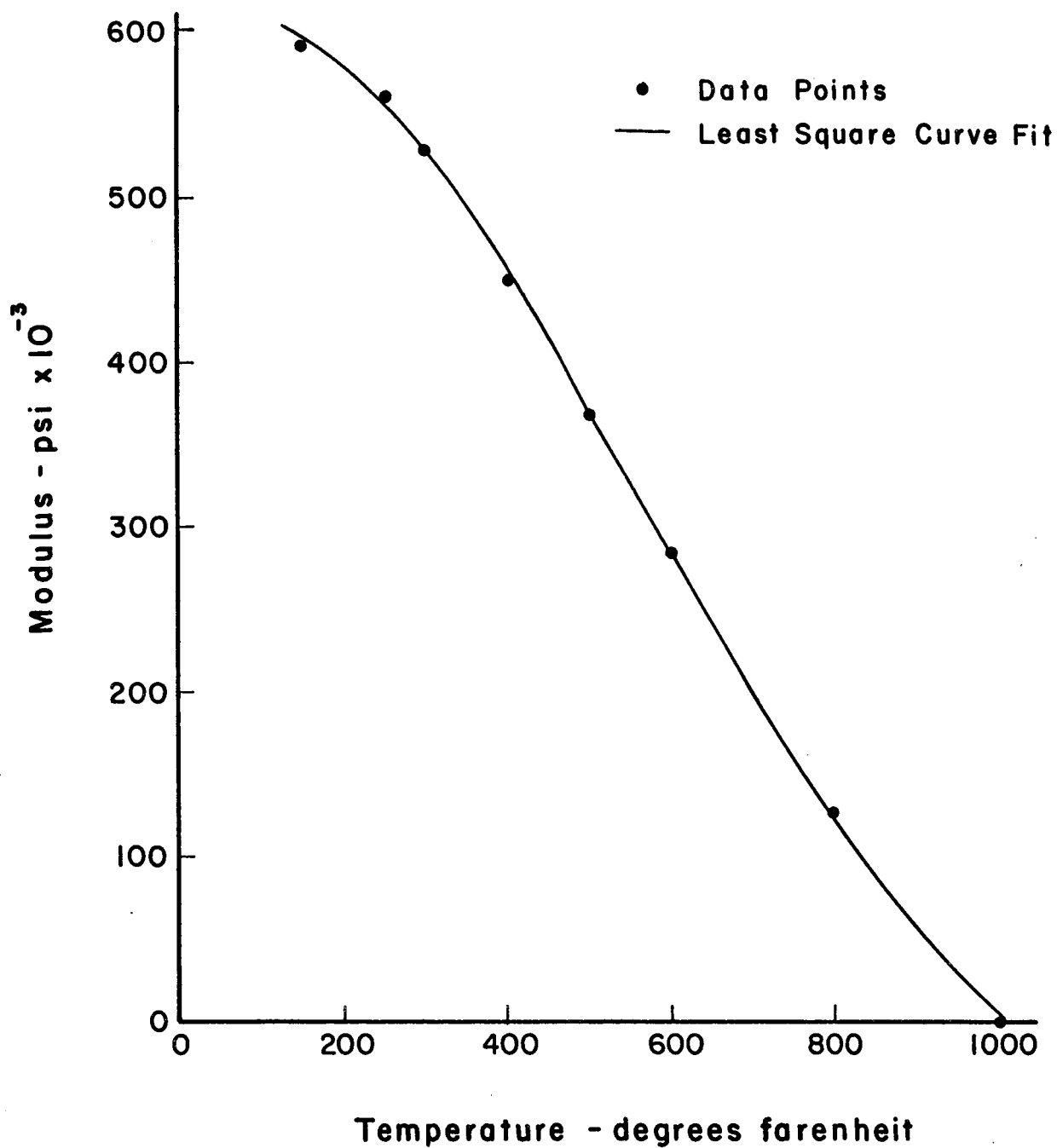


Fig. B1 Example of Least Square Curve Fit

APPENDIX C - MATHEMATICAL MODEL OF SANDWICH SHELL

The sandwich shell substructure is composed of a honeycomb core material and two steel face plates. The orthotropic core material is readily represented by solid triangular rings as indicated in Part IV of this report. However, for the description of the behavior of the face plates, a shell type element is used. The appropriate theory is given in this appendix.

It is assumed that the face plates are idealized by series of truncated cone elements which are connected at mesh points of the finite element system. The cross section of a typical truncated cone element is shown in Figure C1.

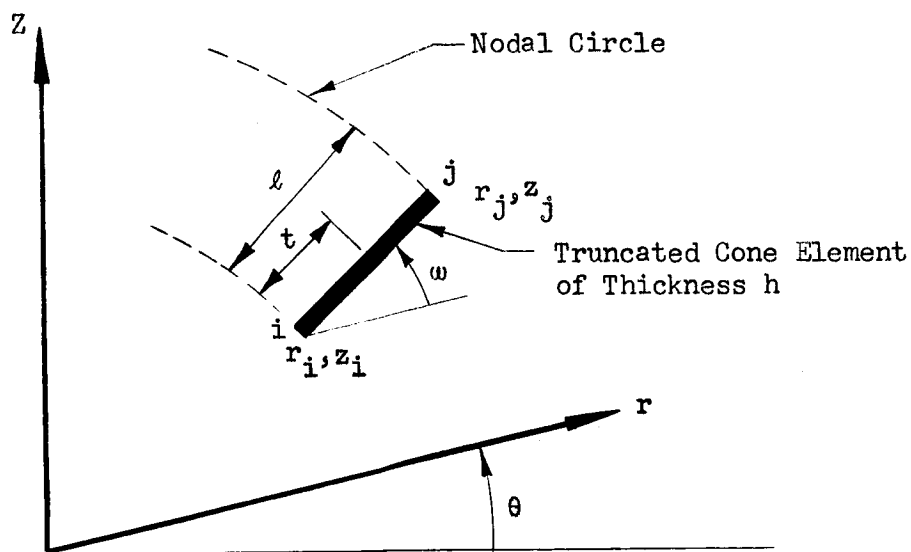


FIGURE C1 - CROSS SECTION OF TRUNCATED CONE ELEMENT

The displacements of the system in the r, θ, z coordinate system are written in the following form:

$$u_r = \sum u_{rn}(r, z) \cos n\theta$$

$$u_z = \sum u_{zn}(r, z) \cos n\theta$$

$$u_\theta = \sum u_{\theta n}(r, z) \sin n\theta$$

where $u_{rn}(r, z)$, $u_{zn}(r, z)$ and $u_{\theta n}(r, z)$ are the two-dimensional displacement functions (Fourier coefficients) associated with the harmonic n .

Within each truncated cone element, the two-dimensional displacement functions are assumed to vary linearly. The displacement at some point t within the element are given in terms of the nodal circle displacements by

$$u_r(t, \theta) = \sum \left[\frac{l-t}{l} u_{rn}^i + \frac{t}{l} u_{rn}^j \right] \cos n\theta \quad (C1)$$

$$u_z(t, \theta) = \sum \left[\frac{l-t}{l} u_{zn}^i + \frac{t}{l} u_{zn}^j \right] \cos n\theta \quad (C2)$$

$$u_\theta(t, \theta) = \sum \left[\frac{l-t}{l} u_{\theta n}^i + \frac{t}{l} u_{\theta n}^j \right] \sin n\theta \quad (C3)$$

Accordingly, the displacement in the t -direction is

$$u_t(t, \theta) = u_r(t, \theta) \cos \omega + u_z(t, \theta) \sin \omega \quad (C4)$$

$$\text{where} \quad \cos \omega = \frac{a}{l}$$

$$\sin \omega = \frac{b}{l}$$

The inplane strains within the truncated cone element are

$$\epsilon_t = \frac{\partial u_t}{\partial t} = \sum \epsilon_{tn} \cos n\theta \quad (C5)$$

$$\epsilon_\theta = \frac{1}{r} \frac{\partial u_\theta}{\partial \theta} + \frac{u_r}{r} = \sum \epsilon_{\theta n} \cos n\theta \quad (C6)$$

$$\gamma_{\theta t} = \frac{1}{r} \frac{\partial u_t}{\partial \theta} + \frac{\partial u_\theta}{\partial t} = \sum \gamma_{\theta tn} \sin n\theta \quad (C7)$$

By combining Equations (C1) through (C7), the inplane strains for harmonic n are expressed in terms of nodal circle displacements by the following matrix equations:

$$\begin{bmatrix} \epsilon_{tn} \\ \epsilon_{\theta n} \\ \gamma_{\theta tn} \end{bmatrix} = \begin{bmatrix} -\frac{a}{l^2} & -\frac{b}{l^2} & 0 & \frac{a}{l^2} & \frac{b}{l^2} & 0 \\ \frac{l-t}{rl} & 0 & \frac{n(l-t)}{rl} & \frac{t}{rl} & 0 & \frac{nt}{rl} \\ -\frac{na}{rl^2}(l-t) - \frac{nb}{rl^2}(l-t) - \frac{1}{l} & -\frac{nat}{rl^2} & -\frac{nbt}{rl^2} & \frac{1}{l} \end{bmatrix} \begin{bmatrix} u_{rn}^i \\ u_{zn}^i \\ u_{\theta n}^i \\ u_{rn}^j \\ u_{tn}^j \\ u_{\theta n}^j \end{bmatrix} \quad (C8)$$

If Equation (C8) is evaluated at the center of the element, the average element strains are given by

$$\begin{bmatrix} \bar{\epsilon}_{tn} \\ \bar{\epsilon}_{\theta n} \\ \bar{\gamma}_{\theta tn} \end{bmatrix} = \begin{bmatrix} -\frac{a}{l^2} & -\frac{b}{l^2} & 0 & \frac{a}{l^2} & \frac{b}{l^2} & 0 \\ \frac{1}{2\bar{r}} & 0 & \frac{n}{2\bar{r}} & \frac{1}{2\bar{r}} & 0 & \frac{n}{2\bar{r}} \\ -\frac{na}{2\bar{r}} & -\frac{nb}{2\bar{r}l} & -\frac{1}{l} & -\frac{na}{2\bar{r}l} & -\frac{nb}{2\bar{r}l} & \frac{1}{l} \end{bmatrix} \begin{bmatrix} u_{rn}^i \\ u_{zn}^i \\ u_{\theta n}^i \\ u_{rn}^j \\ u_{zn}^j \\ u_{\theta n}^j \end{bmatrix} \quad (C9a)$$

where $\bar{r} = (r_i + r_j)/2$

or Equation (C9a) written in symbolic form

$$[\epsilon_n] = [G_n] [u_n] \quad (C9b)$$

From Hooke's law, the stresses within the element are given by

$$\begin{bmatrix} \sigma_{tn} \\ \sigma_{\theta n} \\ \tau_{\theta tn} \end{bmatrix} = \frac{E}{(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} \epsilon_{tn} \\ \epsilon_{\theta n} \\ \gamma_{\theta tn} \end{bmatrix} = \begin{bmatrix} \tau_1 \\ \tau_2 \\ 0 \end{bmatrix} \quad (10a)$$

where $\tau_1 = \tau_2 = \frac{1 + \nu}{1 - \nu} \frac{E \alpha_t}{2} T_n$

Equation (10a) expressed in symbolic form is

$$[\sigma_n] = [C] [\epsilon_n] + [\tau] \quad (10b)$$

From Equation (1.15), the nodal circle forces in terms of the nodal circle displacement for the harmonic n are given by

$$[S_n] = [k_n] [U_n] + [L_n] T_n \quad (C11)$$

where

$$[K_n] = \bar{r} A [G_n]^T [C] [G_n]$$

$$[L_n] = \bar{r} A [G_n]^T [\tau]$$

For the truncated cone element

$$A = h \cdot \ell$$

These forces are included into the overall equilibrium of the system for the harmonic n by the same technique used for the system of triangular rings. When n equals zero, these equations reduce to the axisymmetric case.

APPENDIX D

PROGRAM LISTING - ARBITRARY AXISYMMETRIC STRUCTURES

CAXISY STRESS ANALYSIS OF AXISYMMETRIC SOLIDS

C
C
C

DIMENSION AND COMMON STATEMENTS

```

DIMENSION THDIS(6),THSRC(6)
DIMENSION NPNUM(340),XORD(340),YORD(340),
1DSX(340),DSY(340),XLOAD(340),YLOAD(340),NP(340,10),SXX(340,9),
2SXY(340,9),SYX(340,9),SYY(340,9),NAP(340)
DIMENSION NUME(550),NPI(550),NPJ(550),NPK(550),ET(550),XU(550),
1RO(550),COED(550),DT(550),THERM(550),AJ(550),BJ(550),AK(550),
2BK(550),SIGXX(550),SIGYY(550),SIGXY(550),SLOPE(340)
DIMENSION NPB(340),NFIX(340),LM(3),A(6,6),B(6,6),S(6,6)
COMMON SXX,SXY,SYX,SYY
EQUIVALENCE (SIGXX,RO,NPB), (SIGYY,COED,NFIX), (SIGXY,DT,SLOPE)

```

C
C
C

READ AND PRINT OF DATA

```

KINN=5
KOUT=6
150 READ INPUT TAPE KINN,100
WRITE OUTPUT TAPE KOUT,99
WRITE OUTPUT TAPE KOUT,100
READ INPUT TAPE KINN,1,NUMEL,NUMNP,NUMBC,NCPIN,NOPIN,NCYCM,TOLER,
1XFAC,T1
WRITE OUTPUT TAPE KOUT,101,NUMEL
WRITE OUTPUT TAPE KOUT,102,NUMNP
WRITE OUTPUT TAPE KOUT,103,NUMBC
WRITE OUTPUT TAPE KOUT,104,NCPIN
WRITE OUTPUT TAPE KOUT,105,NOPIN
WRITE OUTPUT TAPE KOUT,106,NCYCM
WRITE OUTPUT TAPE KOUT,107,TOLER
WRITE OUTPUT TAPE KOUT,108,XFAC
READ INPUT TAPE KINN,2,(NUME(N),NPI(N),NPJ(N),NPK(N),ET(N),RO(N),
1XU(N),COED(N),DT(N),N=1,NUMEL)
READ INPUT TAPE KINN,3,(NPNUM(M),XORD(M),YORD(M),XLOAD(M),YLOAD(M)
1,DSX(M),DSY(M),M=1,NUMNP)
IF (T1) 160,155,160
155 WRITE OUTPUT TAPE KOUT,110
WRITE OUTPUT TAPE KOUT,2,(NUME(N),NPI(N),NPJ(N),NPK(N),ET(N),RO(N)
1,XU(N),COED(N),DT(N),N=1,NUMEL)
WRITE OUTPUT TAPE KOUT,111
WRITE OUTPUT TAPE KOUT,109,(NPNUM(M),XORD(M),YORD(M),XLOAD(M),
1YLOAD(M),DSX(M),DSY(M),M=1,NUMNP)

```

C

AXISY089
AXISY090
AXISY091
AXISY092
AXISY093
AXISY094
AXISY095
AXISY096
AXISY097
AXISY098
AXISY099
AXISY100
AXISY101
AXISY102
AXISY103
AXISY104
AXISY105
AXISY106
AXISY107
AXISY108
AXISY109
AXISY110
AXISY111
AXISY112
AXISY113
AXISY114
AXISY115
AXISY116
AXISY117
AXISY118
AXISY119
AXISY120
AXISY121
AXISY122
AXISY123
AXISY124
AXISY125
AXISY126
AXISY127
AXISY128
AXISY129
AXISY130
AXISY131
AXISY132

A(1,4)=0.0
A(1,5)=-BJ(N)
A(1,6)=0.0
A(2,1)=0.0
A(2,2)=AK(N)-AJ(N)
A(2,3)=0.0
A(2,4)=-AK(N)
A(2,5)=0.0
A(2,6)=AJ(N)
A(3,1)=AK(N)-AJ(N)
A(3,2)=BJ(N)-BK(N)
A(3,3)=-AK(N)
A(3,4)=BK(N)
A(3,5)=AJ(N)
A(3,6)=-BJ(N)
C=.66666667*AREA
A(4,1)=C/XORD(I)
A(4,2)=0.0
A(4,3)=C/XORD(J)
A(4,4)=0.0
A(4,5)=C/XORD(K)
A(4,6)=0.0

B. FORM MATRIX (B)

COMM=.25*ET(N)*THICK/(COMM*AREA)
E=XU(N)*COMM
D=(1.-XU(N))*COMM
C=(.5-XU(N))*COMM
B(1,1)=D
B(1,2)=E
B(1,3)=0.0
B(1,4)=E
B(2,1)=E
B(2,2)=D
B(2,3)=0.0
B(2,4)=E
B(3,1)=0.0
B(3,2)=0.0
B(3,3)=C
B(3,4)=0.0
B(4,1)=E
B(4,2)=E
B(4,3)=0.0

B(4,4)=D

C. FORM ELEMENT STIFFNESS MATRIX (A)T*(B)*(A)

DO 182 J=1,6

DO 182 I=1,4

S(I,J)=0.0

DO 182 K=1,4

182 S(I,J)=S(I,J)+B(I,K)*A(K,J)

DO 183 J=1,6

DO 183 I=1,4

183 B(J,I)=S(I,J)

DO 184 J=1,6

DO 184 I=1,6

S(I,J)=0.0

DO 184 K=1,4

184 S(I,J)=S(I,J)+B(I,K)*A(K,J)

3. ADD ELEMENT STIFFNESS TO TOTAL STIFFNESS

LM(1)=NPI(N)

LM(2)=NPJ(N)

LM(3)=NPK(N)

DO 200 L=1,3

DO 200 M=1,3

LX=LM(L)

MX=0

185 MX=MX+1

IF(NP(LX,MX))-LM(M) 190,195,190

190 IF(NP(LX,MX)) 185,195,185

195 NP(LX,MX)=LM(M)

IF (MX-10) 196,702,702

702 WRITE OUTPUT TAPE KOUT,712,(LX)

NTAG=1

196 SXX(LX,MX)=SXX(LX,MX)+S(2*L-1,2*M-1)

SXY(LX,MX)=SXY(LX,MX)+S(2*L-1,2*M)

SYX(LX,MX)=SYX(LX,MX)+S(2*L,2*M-1)

200 SYX(LX,MX)=SYX(LX,MX)+S(2*L,2*M)

5. COMPUTE BODY FORCES

DL=AREA*THICK*RO(N)/3.

I=NPI(N)

J=NPJ(N)

AXISY177
AXISY178
AXISY179
AXISY180
AXISY181
AXISY182
AXISY183
AXISY184
AXISY185
AXISY186
AXISY187
AXISY188
AXISY189
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AXISY191
AXISY192
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AXISY198
AXISY199
AXISY200
AXISY201
AXISY202
AXISY203
AXISY204
AXISY205
AXISY206
AXISY207
AXISY208
AXISY209
AXISY210
AXISY211
AXISY212
AXISY213
AXISY214
AXISY215
AXISY216
AXISY217
AXISY218
AXISY219
AXISY220

```

K=NPK(N)
C=COED(N)*DT(N)
THDIS(1)=C*XORD(I)
THDIS(2)=0.0
THDIS(3)=C*XORD(J)
THDIS(4)=C*BJ(N)
THDIS(5)=C*XORD(K)
THDIS(6)=C*BK(N)
DO 204 L=1,6
  THFRC(L)=0.0
DO 204 M=1,6
  204 THFRC(L)=THFRC(L)+S(L,M)*THDIS(M)
  XLOAD(I)=XLOAD(I)+THFRC(1)
  YLOAD(I)=YLOAD(I)+THFRC(2)-DL
  XLOAD(J)=XLOAD(J)+THFRC(3)
  YLOAD(J)=YLOAD(J)+THFRC(4)-DL
  XLOAD(K)=XLOAD(K)+THFRC(5)
  YLOAD(K)=YLOAD(K)+THFRC(6)-DL
2000 CONTINUE
C
C
C
COUNT OF ADJACENT NODAL POINTS
DO 206 M=1,NUMNP
  MX =1
  205 MX=MX+1
  IF (NP(M,MX)) 206,206,205
  206 NAP(M)=MX-1
C
C
C
INVERSION OF NODAL POINT STIFFNESS
DO 210 M=1,NUMNP
  COMM=SXX(M,1)*SYY(M,1)-SXY(M,1)*SYX(M,1)
  TEMP=SYY(M,1)/COMM
  SYY(M,1)=SXX(M,1)/COMM
  SXX(M,1)=TEMP
  SXY(M,1)=-SXY(M,1)/COMM
  210 SYX(M,1)=-SYX(M,1)/COMM
C
C
C
MODIFICATION OF BOUNDARY FLEXIBILITIES
WRITE OUTPUT TAPE KOUT,112
READ INPUT TAPE KINN,4,(NPB(L),NFIX(L),SLOPE(L),L=1,NUMBC)
WRITE OUTPUT TAPE KOUT,4,(NPB(L),NFIX(L),SLOPE(L),L=1,NUMBC)
DO 240 L=1,NUMBC

```

WRITE OUTPUT TAPE KOUT,120,NCYCLE,SUM
 305 IF (SUM-TOLER) 400,400,310
 310 IF(NCYCM-NCYCLE) 400,400,315
 315 IF (NCYCLE-NUMOPT) 244,320,320
 320 NUMOPT=NUMOPT+NOPIN

PRINT OF NODAL POINT DISPLACEMENTS

400 WRITE OUTPUT TAPE KOUT,99
 WRITE OUTPUT TAPE KOUT,100
 WRITE OUTPUT TAPE KOUT,121
 WRITE OUTPUT TAPE KOUT,122,(NPNUM(M),DSX(M),DSY(M),M=1,NUMNP)

CALCULATION AND PRINT OF ELEMENT STRESSES

WRITE OUTPUT TAPE KOUT,123

DO 420 N=1,NUMEL

I=NPI(N)

J=NPJ(N)

K=NPK(N)

1. COMPUTE ELEMENT STRAINS

C=AJ(N)*BK(N)-BJ(N)*AK(N)
 EPX=((BJ(N)-BK(N))*DSX(I)+BK(N)*DSX(J)-BJ(N)*DSX(K))/C
 EPY=((AK(N)-AJ(N))*DSY(I)-AK(N)*DSY(J)+AJ(N)*DSY(K))/C
 EPZ=(DSX(I)+DSX(J)+DSX(K))/(XORD(I)+XORD(J)+XORD(K))
 GAM=((AK(N)-AJ(N))*DSX(I)-AK(N)*DSX(J)+AJ(N)*DSX(K)
 1 + (BJ(N)-BK(N))*DSY(I)+BK(N)*DSY(J)-BJ(N)*DSY(K))/C

2. COMPUTE ELEMENT STRESSES

E=ET(N)*XU(N)/((1.+XU(N))*(1.-2.*XU(N)))
 D=E*(1.-XU(N))/XU(N)
 C=E*(1.-2.*XU(N))/(2.*XU(N))
 X= D*EPX + E*EPY + E*EPZ +THERM(N)
 Y= E*EPX + D*EPY + E*EPZ +THERM(N)
 Z= E*EPX + E*EPY + D*EPZ +THERM(N)
 XY=C*GAM
 SIGXX(N)=X
 SIGYY(N)=Y
 SIGXY(N)=XY

3.COMPUTE PRINCIPAL STRESSES

AXISY265
 AXISY266
 AXISY267
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 AXISY269
 AXISY270
 AXISY271
 AXISY272
 AXISY273
 AXISY274
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 AXISY276
 AXISY277
 AXISY278
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 AXISY306
 AXISY307
 AXISY308


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AXISY221
AXISY222
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AXISY251
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AXISY255
AXISY256
AXISY257
AXISY258
AXISY259
AXISY260
AXISY261
AXISY262
AXISY263
AXISY264

M=NPB(L)
NP(M,1)=0
IF(NFIX(L)-1) 225,220,215
215 C=(SXX(M,1)*SLOPE(L)-SXY(M,1))/(SYX(M,1)*SLOPE(L)-SYY(M,1))
R=1-C*SLOPE(L)
SXX(M,1)=(SXX(M,1)-C*SXX(M,1))/R
SXY(M,1)=(SXY(M,1)-C*SXY(M,1))/R
SYX(M,1)=SXX(M,1)*SLOPE(L)
SYY(M,1)=SXY(M,1)*SLOPE(L)
GO TO 240
220 SYY(M,1)=SYY(M,1)-SYX(M,1)*SXY(M,1)/SXX(M,1)
GO TO 230
225 SYY(M,1)=0.0
230 SXX(M,1)=0.0
235 SXY(M,1)=0.0
SYX(M,1)=0.0
240 CONTINUE

C
C
C
      ITERATION ON NODAL POINT DISPLACEMENTS
      IF (NTAG) 150,243,150
243 WRITE OUTPUT TAPE KOUT,119
244 SUM=0.0
DO 290 M=1,NUMNP
  NUM=NAP(M)
  IF (SXX(M,1)+SYY(M,1)) 275,290,275
275 FRX=XLOAD(M)
  FRY=YLOAD(M)
DO 280 L=2,NUM
  N=NP(M,L)
  FRX=FRX-SXX(M,L)*DSX(N)-SXY(M,L)*DSY(N)
280 FRY=FRY-SYX(M,L)*DSX(N)-SYY(M,L)*DSY(N)
  DX=SXX(M,1)*FRX+SXY(M,1)*FRY-DSX(M)
  DY=SYX(M,1)*FRX+SYY(M,1)*FRY-DSY(M)
  DSX(M)=DSX(M)+XFAC*DX
  DSY(M)=DSY(M)+XFAC*DY
285 SUM=SUM+(ABSF(DX)+ABSF(DY))/(SXX(M,1)+SYY(M,1))
290 CONTINUE

C
C
C
      CYCLE COUNT AND PRINT CHECK
      NCYCLE=NCYCLE +1
      IF (NCYCLE-NUMPT) 305,300,300
300 NUMPT=NUMPT+NCPIN

```


APPENDIX E

PROGRAM LISTING - AXISYMMETRIC HEAT SHIELDS

Report No. F5654-01

CASHS

ANALYSIS OF AXISYMMETRIC HEAT SHIELDS

ASHS0001
ASHS0002
ASHS0003
ASHS0004
ASHS0005
ASHS0006
ASHS0007
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ASHS0036
ASHS0037
ASHS0038
ASHS0039
ASHS0040
ASHS0041
ASHS0042

DIMENSION

1 XX(5),YY(5),S(10,10),P(10),X(40,10),Y(40,10),U(40,10),V(40,10),
1 SS(800,24),RR(800),KODE(40,10),HED(12),T(40,10),CC(8,3)
4 ,A(4,4),B(4,4)

COMMON KINN,KOUT,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,
1 ES,XS,AS,TS,ESH,XSH,ASH,TSH,
1 HED,X,Y,T,U,V,CC,XX,YY,S,P,KODE,RR,SS

READ AND PRINT OF DATA

50 KINN=5
KOUT=6

CALL INPUT

MX=2*MMAX
NS=NMAX*MMAX
MM=MMAX-1
NN=NMAX-1
MBAND=2*(MMAX+2)
NEQ=2*NS

FORMATION OF STIFFNESS ARRAY

DO 175 I=1,NEQ
DO 170 J=1,MBAND
170 SS(I,J)=0.0
175 RR(I)=0.0

DO 200 N=1,NN
DO 200 M=1,MM
DO 180 I=1,10
P(I)=0.0
DO 180 J=1,10
180 S(I,J)=0.0
XX(1)=X(N,M)
XX(2)=X(N,M+1)
XX(3)=X(N+1,M)
XX(4)=X(N+1,M+1)

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ASHS0115
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ASHS0123
ASHS0124
ASHS0125
ASHS0126

```

C
A(2,3)=-A(2,1)
A(2,4)=-A(2,2)
B(1,1)=(ESH/(1.-XSH**2))*TSH*XL*(RI+RJ)/2.
B(1,2)=B(1,1)*XSH
B(2,1)=B(1,2)
B(2,2)=B(1,1)
C
DO 470 I=1,2
DO 470 J=1,4
S(I,J)=0.0
DO 470 K=1,2
470 S(I,J)=S(I,J)+B(I,K)*A(K,J)
C
DO 480 I=1,4
DO 480 J=1,4
B(I,J)=0.0
DO 480 K=1,2
480 B(I,J)=B(I,J)+A(K,I)*S(K,J)
C
AT=ASH*(T(N,M)+T(N+1,M))/2.
A(1)=AT*RI
A(2)=AT*ZI
A(3)=AT*RJ
A(4)=AT*ZJ
C
DO 485 I=1,4
P(I)=0.0
DO 485 K=1,4
485 P(I)=P(I)+B(I,K)*A(K)
C
DO 495 I=1,2
II=((N-1)*MMAX+MB-1)*2+I
KK=II+MX
RR(II)=RR(II)+P(I)
RR(KK)=RR(KK)+P(I+2)
DO 490 J=1,2
JJ=J-I+1
SS(II,JJ)=SS(II,JJ)+B(I,J)
490 SS(KK,JJ)=SS(KK,JJ)+B(I+2,J+2)
DO 495 J=1,2
JJ=J-I+1+MX

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ASHS0168

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495 SS(II,JJ)=SS(II,JJ)+B(I,J+2)
C
500 CONTINUE
C
READ IN SPECIFIED RADIAL AND AXIAL LOADS
C
IF(NUMLD) 505, 202, 505
C
505 WRITE OUTPUT TAPE KOUT, 2000
C
DO 550 I=1,NUMLD
READ INPUT TAPE KINN, 1001,N,M,PR,PZ
WRITE OUTPUT TAPE KOUT, 2001, N,M,PR,PZ
K = 2*( (N-1)*MMAX+M ) -1
RR(K) = RR(K) + PR
550 RR(K+1) = RR(K+1) + PZ
C
BOUNDARY CONDITIONS
C
202 DO 60 N=1,NMAX
DO 60 M=1,MMAX
60 CODE(N,M)=0
C
FIX EDGE
C
DO 205 M=1,MMAX
205 CODE(1,M)=1
C
READ IN ADDITIONAL BOUNDARY CONDITIONS
C
WRITE OUTPUT TAPE KOUT, 2002
C
DO 208 I=1,NUMBC
READ INPUT TAPE KINN,1000,NNN,MMM,KKK
WRITE OUTPUT TAPE KOUT, 1000, NNN,MMM,KKK
208 CODE(NNN,MMM)=KKK
C
KK=0
DO 250 N=1,NMAX
DO 250 M=1,MMAX
IF (CODE(N,M)) 210,250,210
210 LL=1

```

ASHS0169
 ASHS0170
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 ASHS0199
 ASHS0200
 ASHS0201
 ASHS0202
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 ASHS0204
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 ASHS0206
 ASHS0207
 ASHS0208
 ASHS0209
 ASHS0210

```

LH=2
IF (KODE(N,M)-2) 211,212,213
211 LH=1
    GO TO 213
212 LL=2
213 DO 245 L=LL,LH
    II=KK+L
    DO 240 J=1,MBAND
      SS(II,J)=0.0
      III=II+1-J
      IF (III) 240,240,230
230 SS(III,J)=0.0
240 CONTINUE
    SS(II,1)=1.
245 RR(II)=0.0
250 KK=KK+2
C
C
C
C
C
    SOLVE FOR DISPLACEMENTS
    CALL SYMSOL(SS,RR,NEQ,MBAND)
    K=1
    DO 400 N=1,NMAX
      DO 400 M=1,MMAX
        U(N,M)=RR(K)
        V(N,M)=RR(K+1)
        400 K=K+2
C
    WRITE OUTPUT TAPE KOUT,2003
    CALL PRINTM(U,NMAX,MMAX,40)
    WRITE OUTPUT TAPE KOUT,2004
    CALL PRINTM(V,NMAX,MMAX,40)
C
    CALCULATE AND PRINT STRESSES
    CALL STRESS
    GO TO 50
    FORMAT STATEMENTS
C
C
C
C
C
  
```

ASHS0211
 ASHS0212
 ASHS0213
 ASHS0214
 ASHS0215
 ASHS0216
 ASHS0217
 ASHS0218
 ASHS0219
 ASHS0220

1000 FORMAT (3I5)
 1001 FORMAT (2I5, 2F10.3)
 2000 FORMAT (46H1 POINTS WITH SPECIFIED RADIAL AND AXIAL LOADS //
 113X 26HN M R-LOAD Z-LOAD)
 2001 FORMAT (9X 2I5, 2F10.3)
 2002 FORMAT (44H1 POINTS WITH ADDITIONAL BOUNDARY CONDITIONS //
 116H N M CODE)
 2003 FORMAT (16H1R-DISPLACEMENTS)
 2004 FORMAT (16H1Z-DISPLACEMENTS)
 END

C
 Report No. F5654-01

```

    SUBROUTINE INPUT
      COMMON AND DIMENSION STATEMENTS
      DIMENSION HED(12),R(40,10),Z(40,10),T(40,10),CC(5,3),TA(40),
    1 TT(50),A(50,5),B(5,5),D(5,3),EE(50,3),U(40,10),V(40,10)
    2,KR(5)
      COMMON KINN,KOUT,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,
    1 ES,XS,AS,TS,ESH,XSH,ASH,TSH,
    1 HED,R,Z,T,U,V,CC,TA,TT,A,B,D,EE
      READ AND PRINT INPUT DATA
      READ INPUT TAPE KINN,1000,
    1HED,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,ES,XS,AS,TS,ESH,XSH,ASH,TSHASHS0236
      WRITE OUTPUT TAPE KOUT,2000,
    1HED,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR
      CHECK THE SIZE OF NMAX, MMAX, NMT AND LOCATION OF MB. CHECK FOR
      ADDITIONAL BOUNDARY CONDITIONS. IF ANY INPUT ERRORS EXIT.
      DO 1 I=1,5
    1 KR(I) = 0
      IF (40-NMAX) 5, 10, 10
    5 KR(1) = 1
    10 IF (10-MMAX) 15, 20, 20
    15 KR(2) = 2
    20 IF (MMAX-MB) 25, 25, 30
    25 KR(3) = 3
    30 IF (50-NMT) 35, 40, 40
    35 KR(4) = 4
    40 IF (NUMBC) 45, 45, 50
    45 KR(5) = 6
      50 DO 65 I=1,5
      IF(KR(I)) 60, 65, 60
    60 WRITE OUTPUT TAPE KOUT, 2011, KR(I)
      KR = 1
    65 CONTINUE
      IF(KR) 70, 75, 70
    70 CALL EXIT
    75 WRITE OUTPUT TAPE KOUT, 2006
  
```

C
 Page E7

ASHS0221
 ASHS0222
 ASHS0223
 ASHS0224
 ASHS0225
 ASHS0226
 ASHS0227
 ASHS0228
 ASHS0229
 ASHS0230
 ASHS0231
 ASHS0232
 ASHS0233
 ASHS0234
 ASHS0235
 ASHS0236
 ASHS0237
 ASHS0238
 ASHS0239
 ASHS0240
 ASHS0241
 ASHS0242
 ASHS0243
 ASHS0244
 ASHS0245
 ASHS0246
 ASHS0247
 ASHS0248
 ASHS0249
 ASHS0250
 ASHS0251
 ASHS0252
 ASHS0253
 ASHS0254
 ASHS0255
 ASHS0256
 ASHS0257
 ASHS0258
 ASHS0259
 ASHS0260
 ASHS0261
 ASHS0262
 ASHS0263
 ASHS0264
 ASHS0265
 ASHS0266

```

WRITE OUTPUT TAPE KOUT,2008, ES,XS,AS,TS
WRITE OUTPUT TAPE KOUT,2007
WRITE OUTPUT TAPE KOUT,2008, ESH,XSH,ASH,TSH
READ INPUT TAPE KINN,1001.
1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)
WRITE OUTPUT TAPE KOUT,2001,
1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)

LEAST SQUARE EVALUATION OF MATERIAL PROPERTIES

READ INPUT TAPE KINN,1002.
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)
WRITE OUTPUT TAPE KOUT, 2009
WRITE OUTPUT TAPE KOUT,2002,
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)

DO 100 I=1,NMT
A(I,1)=1.0
A(I,2)=TT(I)
A(I,3)=TT(I)*TT(I)
A(I,4)=A(I,3)*TT(I)
100 A(I,5)=A(I,4)*TT(I)

CALL LEAST(A,EE,B,D,CC,NMT,5,3)

WRITE OUTPUT TAPE KOUT, 2010
WRITE OUTPUT TAPE KOUT,2002,
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)

GENERATE MESH

MC = MB+1
MBB = MB-1
SHL = FLOATF(MBB)
ABL = FLOATF(MMAX-MB)
DO 200 N=1,NMAX

CHECK FOR END POINTS

NL = N-1
NH=N+1
IF(NL)160,150,160
150 SS = 0.
CX = 1.
GO TO 185
160 IF(N-NMAX)180,170,180

```

ASHS0267
 ASHS0268
 ASHS0269
 ASHS0270
 ASHS0271
 ASHS0272
 ASHS0273
 ASHS0274
 ASHS0275
 ASHS0276
 ASHS0277
 ASHS0278
 ASHS0279
 ASHS0280
 ASHS0281
 ASHS0282
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 ASHS0284
 ASHS0285
 ASHS0286
 ASHS0287
 ASHS0288
 ASHS0289
 ASHS0290
 ASHS0291
 ASHS0292
 ASHS0293
 ASHS0294
 ASHS0295
 ASHS0296
 ASHS0297
 ASHS0298
 ASHS0299
 ASHS0300
 ASHS0301
 ASHS0302
 ASHS0303
 ASHS0304
 ASHS0305
 ASHS0306
 ASHS0307
 ASHS0308
 ASHS0309
 ASHS0310
 ASHS0311
 ASHS0312

```

170  NH=N
C
C      COMPUTE SIN AND COS AT EACH POINT ON BOND LINE
C
180  XX = R( NH,MB) - R(NL,MB)
    YY = Z( NH ,MB) - Z(NL,MB)
    ZZ = SQRTF(XX**2 + YY**2)
    SS = YY/ZZ
    CX = XX/ZZ
C
C      COMPUTE COORDINATES OF POINTS IN CONSTANT THICKNESS SHELL
C
185  DO 190 M=1,MBB
    TT = TS*FLOATF(MB-M)/SHL
    T(N,M) = T(N,MB)
    R(N,M) = R(N,MB) + TT*SS
    Z(N,M) = Z(N,MB) - TT*CX
190  COMPUTE COORDINATES OF POINTS IN VARIABLE THICKNESS ABLATER
C
C      DO 200 M=MC,MMAX
    TT = TA(N)*FLOATF(M-MB)/ABL
    T(N,M) = T(N,MB) + (TE - T(N,MB)) * (TT/TA(N))**2
    R(N,M) = R(N,MB) - TT*SS
    Z(N,M) = Z(N,MB) + TT*CX
200
C
C      DUMP OUT R-ORDINATE TABLE AND Z-ORDINATE TABLE
C
C      WRITE OUTPUT TAPE KOUT,2003
    CALL PRINTM (R,NMAX,MMAX,40)
    WRITE OUTPUT TAPE KOUT,2004
    CALL PRINTM (Z,NMAX,MMAX,40)
    WRITE OUTPUT TAPE KOUT,2005
    CALL PRINTM (T,NMAX,MMAX,40)
    RETURN
C
C      FORMAT STATEMENTS
C
1000  FORMAT (12A6/6I5,2F10.2/4F10.2/4F10.2)
1001  FORMAT (4F10.2)
1002  FORMAT (4F10.2)
2000  FORMAT (1H1 12A6/
      1 33HNUMBER OF POINTS ALONG LENGTH--- I3/
      2 33H NUMBER OF POINTS THRU THICKNESS- I3/
      3 33H LOCATION OF BOND LINE----- I3/
      4 33H NUMBER OF PROPERTY CARDS----- I3/
      5 33H NUMBER OF POINTS WITH R,Z LOADS- I3/

```

6 33H ADDITIONAL BOUNDARY CONDITIONS-- I3/
 7 33H SURFACE TEMPERATURE OF ABLATOR-- F6.0/
 8 33H ZERO STRESS TEMPERATURE----- F6.0)
 2001 FORMAT (15H1 R-ORDINATE 5X 10HZ-ORDINATE 5X 10HBOND TEMP. 3X
 1 17HABLATOR THICKNESS / (3F15.3,1F20.4))
 2002 FORMAT (15H0 TEMPERATURE 5X 10HMODULUS A 5X 9HMODULUS B 1X
 120H COEFF. OF EXPANSION / (3F15.0,1E20.5))
 2003 FORMAT (14H1 R-ORDINATES)
 2004 FORMAT (14H1 Z-ORDINATES)
 2005 FORMAT (14H1 TEMPERATURE)
 2006 FORMAT (37H0PROPERTIES OF SANDWICH CORE MATERIAL)
 2007 FORMAT (30H0PROPERTIES OF SANDWICH PLATES)
 2008 FORMAT
 1(28H MODULUS OF ELASTICITY----- F10.0/
 2 28H POISSONS RATIO----- F10.4/
 3 28H COEFFICIENT OF EXPANSION-- F10.8/
 4 28H THICKNESS----- F10.4)
 2009 FORMAT (25H1 MATERIAL PROPERTY DATA)
 2010 FORMAT (1H1 5X 55HLEAST SQUARE EVALUATION OF ABOVE MATERIAL PROPEASHS0377
 1RTY DATA)
 2011 FORMAT (49H0 ERROR IN ABOVE INPUT DATA. CHECK VALUE ON LINE I3)ASHS0379
 END
 ASHS0359
 ASHS0360
 ASHS0361
 ASHS0362
 ASHS0363
 ASHS0364
 ASHS0365
 ASHS0366
 ASHS0367
 ASHS0368
 ASHS0369
 ASHS0370
 ASHS0371
 ASHS0372
 ASHS0373
 ASHS0374
 ASHS0375
 ASHS0376
 ASHS0377
 ASHS0378
 ASHS0379
 ASHS0380

```

SUBROUTINE TRIST(IX,JX,KX,TT,NNN)
  DIMENSION XX(5),YY(5),S(10,10),P(10),A(6,6),B(6,6),C(6,6),LM(3)
  1  , HED(12),X(40,10),Y(40,10),T(40,10),CC(5,3),EE(6),U(40,10)
  3  ,V(40,10)

  COMMON KINN,KOUT,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,
  1  ES,XS,AS,TS,ESH,XSH,ASH,TSH,
  1  HED,X,Y,T,U,V,CC,XX,YY,S,P,C

  INITIALIZATION
    AJ=XX(JX)-XX(IX)
    AK=XX(KX)-XX(IX)
    BJ=YY(JX)-YY(IX)
    BK=YY(KX)-YY(IX)
    TEMP=AJ*BK-BJ*AK
    AREA=TEMP/2.
    THICK=(XX(IX)+XX(JX)+XX(KX))/3.
    DO 50 I=1,36
      A(I)=0.0
      B(I)=0.0
    50

  CALCULATE MATERIAL PROPERTIES
    DO 60 I=1,3
      60 EE(I)=CC(1,I)+TT*(CC(2,I)+TT*(CC(3,I)+TT*(CC(4,I)+TT*CC(5,I))))
      IF (NNN) 70,70,80
      70 EP=EE(I)
      ALP=EE(3)
      XNU=0.3155+0.000174*TT
      GO TO 90
      80 EP=ES
      ALP=AS
      XNU=XS
      90 AT=ALP*(TT-TR)
      COMM=EP/(1.-2.*XNU)
      TE=COMM*AT
      COMM=COMM/(1.+XNU)
      FORM MATRIX (B)

```

ASHS0423
ASHS0424
ASHS0425
ASHS0426
ASHS0427
ASHS0428
ASHS0429
ASHS0430
ASHS0431
ASHS0432
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ASHS0434
ASHS0435
ASHS0436
ASHS0437
ASHS0438
ASHS0439
ASHS0440
ASHS0441
ASHS0442
ASHS0443
ASHS0444
ASHS0445
ASHS0446
ASHS0447
ASHS0448
ASHS0449
ASHS0450
ASHS0451
ASHS0452
ASHS0453
ASHS0454
ASHS0455
ASHS0456
ASHS0457
ASHS0458
ASHS0459
ASHS0460
ASHS0461
ASHS0462
ASHS0463
ASHS0464

E=XNU*COMM
D=(1.-XNU)*COMM
C=(.5-XNU)*COMM
B(1,1)=D
B(1,2)=E
B(1,4)=E
B(2,1)=E
B(2,2)=D
B(2,4)=E
B(3,3)=C
B(4,1)=E
B(4,2)=E
B(4,4)=D

C
C
C

FORM MATRIX (A)

A(1,1)=BJ-BK
A(1,3)=BK
A(1,5)=-BJ
A(2,2)=AK-AJ
A(2,4)=-AK
A(2,6)=AJ
A(3,1)=AK-AJ
A(3,2)=BJ-BK
A(3,3)=-AK
A(3,4)=BK
A(3,5)=AJ
A(3,6)=-BJ
C=.333333*TEMP/THICK
A(4,1)=C
A(4,3)=C
A(4,5)=C

DO 95 I=1,36
95 A(I)=A(I)/TEMP

C
C
C

FORM TRIANGULAR ELEMENT STIFFNESS MATRIX (A)T(B)(A)

COMM=THICK*AREA
DO 100 L=1,4
DO 100 M=1,6
C(L,M)=0.0
DO 100 N=1,4

```

100 C(L,M)=C(L,M)+B(L,N)*A(N,M)
    DO 210 L=1,6
    DO 210 M=1,6
    B(L,M)=0.0
    DO 200 N=1,4
    B(L,M)=B(L,M)+A(N,L)*C(N,M)
210 B(L,M)=B(L,M)*COMM
C
C
C
    COMPUTE TEMPERATURE FORCES
    A(1)=AT*XX(IX)
    A(2)=0.0
    A(3)=AT*XX(JX)
    A(4)=AT*BJ
    A(5)=AT*XX(KX)
    A(6)=AT*BK
C
    DO 220 I=1,6
    EE(I)=0.0
    DO 220 K=1,6
    EE(I)=EE(I)+B(I,K)*A(K)
220
C
C
C
    ADD ELEMENT STIFFNESS TO QUADRILATERAL STIFFNESS
    LM(1)=2*IX-2
    LM(2)=2*JX-2
    LM(3)=2*KX-2
    DO 250 I=1,3
    DO 250 K=1,2
    II=LM(I)+K
    KK=2*I-2+K
    P(II)=P(II)+EE(KK)
    DO 250 J=1,3
    DO 250 L=1,2
    JJ=LM(J)+L
    LL=2*J-2+L
    S(II,JJ)=S(II,JJ)+B(KK,LL)
250
C
C
C
    RETURN
    END

```

ASH50506

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```

      SIG(I)=0.0
      DO 150 K=1,6
        150 SIG(I)=SIG(I)+C(I,K)*P(K)
      C
      CALL TRIST(1,3,4,TT,NNN)
      C
      P(1)=U(N,M)
      P(2)=V(N,M)
      P(3)=U(N+1,M)
      P(4)=V(N+1,M)
      P(5)=U(N+1,M+1)
      P(6)=V(N+1,M+1)
      C
      DO 170 I=1,4
        DO 160 K=1,6
          160 SIG(I)=SIG(I)+C(I,K)*P(K)
          170 SIG(I)=SIG(I)/2.0
        C
        SIGXX(N,M)=SIG(1)-TE
        SIGYY(N,M)=SIG(2)-TE
        SIGXY(N,M)=SIG(3)
        SIGTT(N,M)=SIG(4)-TE
      C
      COST = .5*( X(N,MB) + X(N+1,MB) - X(N,1) - X(N+1,1) ) / TS
      SINT = .5*( Y(N,MB) + Y(N+1,MB) - Y(N,1) - Y(N+1,1) ) / TS
      COS2=COST**2
      SIN2=SINT**2
      TT=2.0*SIGXY(N,M)*SINT*COST
      SIGNN(N,M)=SIGXX(N,M)*COS2+SIGYY(N,M)*SIN2+TT
      SIGSS(N,M)=SIGXX(N,M)*SIN2+SIGYY(N,M)*COS2-TT
      SIGNS(N,M)=(SIGYY(N,M)-SIGXX(N,M))*SINT*COST+SIGXY(N,M)*
        1 (COS2-SIN2)
      200 CONTINUE
      C
      PRINT STRESSES
      C
      WRITE OUTPUT TAPE KOUT,1006
      CALL PRINTM(SIGTT,NN,MM,40)
      WRITE OUTPUT TAPE KOUT,1007
      CALL PRINTM(SIGXX,NN,MM,40)
      WRITE OUTPUT TAPE KOUT,1008
      CALL PRINTM(SIGYY,NN,MM,40)

```



```

      COMM = EE(2)/(1.-XNU**2)
      BSIGS(N) = COMM*(EPS+XNU*EPT)
300  BSIGT(N) = COMM*(EPT+XNU*EPS)
      WRITE OUTPUT TAPE KOUT, 1014, (N, BSIGS(N), BSIGT(N), SIGNN(N,MB),
1  SIGNS(N,MB), N=1,NN )
C
250  RETURN
C
C
      FORMAT STATEMENTS
C
1006  FORMAT (10H1 T-STRESS)
1007  FORMAT (10H1 R-STRESS)
1008  FORMAT (10H1 Z-STRESS)
1009  FORMAT (10H1RZ-STRESS)
1011  FORMAT (10H1 S-STRESS)
1013  FORMAT ( 1H1 42X 27HSTRESSES IN SANDWICH PLATES /
1      27X 11HLOWER PLATE 39X 11HUPPER PLATE /
2      20X 5HMERID 15X 4HHOOP 25X 5HMERID 15X 4HHOOP /
3      4X 1HN 14X 6HSTRESS 14X 6HSTRESS 24X 6HSTRESS 14X 6HSTRESS //
4      ( 15, 2F20.0, 10X 2F20.0) )
1014  FORMAT (1H1 11X 22HSTRESSES IN BOND LAYER //
145H  N      MERID      HOOP      NORMAL      SHEAR //
2(15, 4F10.1) )
      END

```

```

C
SUBROUTINE LEAST(A,B,C,D,E,N,M,L)
C
C
C
C
DIMENSION A(50,5),B(50,3),C(5,5),D(5,3),E(5,3)
C
C
C
C
C)=(A)*T*(A) AND (D)=(A)*T*(B)
C
C
C
C
DO 200 I=1,M
DO 100 J=1,M
C(I,J)=0.0
DO 100 K=1,N
100 C(I,J)=C(I,J)+A(K,I)*A(K,J)
DO 200 J=1,L
D(I,J)=0.0
DO 200 K=1,N
200 D(I,J)=D(I,J)+A(K,I)*B(K,J)
C
C
C
C
INVERT (C)
C
CALL INVERT(C,M,5,B,E)
C
C
C
C
C)=(C)*(D)
C
C
C
C
DO 300 I=1,M
DO 300 J=1,L
E(I,J)=0.0
DO 300 K=1,M
300 E(I,J)=E(I,J)+C(I,K)*D(K,J)
C
C
C
C
CHECK RESULTS (B)=(A)*(E)
C
C
C
C
DO 400 I=1,N
DO 400 J=1,L
B(I,J)=0.0
DO 400 K=1,M
400 B(I,J)=B(I,J)+A(I,K)*E(K,J)
C
C
C
C
RETURN
C
C
END

```

ASHS0783
 ASHS0784
 ASHS0785
 ASHS0786
 ASHS0787
 ASHS0788
 ASHS0789
 ASHS0790
 ASHS0791
 ASHS0792
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 ASHS0819
 ASHS0820
 ASHS0821
 ASHS0822
 ASHS0823
 ASHS0824

```

SUBROUTINE SYMSOL (A,B,NN,MM)
  C
  DIMENSION A(800,24),B(800),C(24)
  C
  N = 0
  100 N = N+1
  C
  REDUCE N TH EQUATION
  C
  1. DIVIDE RIGHT SIDE BY DIAGONAL ELEMENT
  C
  B(N) = B(N) / A(N,1)
  C
  2. CHECK FOR LAST EQUATION
  C
  IF(N-NN) 150,300,150
  C
  3. DIVIDE N TH EQUATION BY DIAGONAL ELEMENT
  C
  150 DO 200 K=2,MM
    C(K) = A(N,K)
  200 A(N,K) = A(N,K) / A(N,1)
  C
  4. REDUCE REMAINING EQUATIONS
  C
  DO 260 L=2,MM
    I = N+L-1
    IF(NN-I) 260,240,240
  240 J=0
    DO 250 K=L,MM
      J=J+1
    250 A(I,J) = A(I,J) - C(L) * A(N,K)
    B(I) = B(I) - C(L) * B(N)
  260 CONTINUE
    GO TO 100
  C
  BACK SUBSTITUTION
  C
  300 N = N-1
  C
  1. CHECK FOR FIRST EQUATION
  C

```

ASHS0825
 ASHS0826
 ASHS0827
 ASHS0828
 ASHS0829
 ASHS0830
 ASHS0831
 ASHS0832
 ASHS0833
 ASHS0834
 ASHS0835
 ASHS0836
 ASHS0837
 ASHS0838

```

IF(N) 350,500,350
C
C      2. CALCULATE UNKNOWN B(N)
C
350 DO 400 K=2,MM
L = N+K-1
IF(NN-L) 400,370,370
370 B(N) = B(N) - A(N,K) * B(L)
400 CONTINUE
GO TO 300
C
500 RETURN
C
END
  
```

```

SUBROUTINE INVERT(A,NN,N,M,C)
      GENERAL MATRIX INVERSION SUBROUTINE

      DIMENSION A(1),M(1),C(1)

      DO 90 I=1,NN
        90 M(I)=-1
      C

      DO 140 I=1,NN
        LOCATE LARGEST ELEMENT
        D=0.0
        DO 112 L=1,NN
          IF (M(L)) 100,100,112
        100 J=L
        DO 110 K=1,NN
          IF (M(K)) 103,103,108
          103 IF (ABSF(D)-ABSF(A(J))) 105,105,108
          105 LD=L
          KD=K
          D=A(J)
          108 J=J+N
          110 CONTINUE
          112 CONTINUE
        C
        INTERCHANGE ROWS
        TEMP=-M(LD)
        M(LD)=M(KD)
        M(KD)=TEMP
        L=LD
        K=KD
        DO 114 J=1,NN
          C(J)=A(L)
          A(L)=A(K)
          A(K)=C(J)
          L=L+N
          114 K=K+N
        C
        DIVIDE COLUMN BY LARGEST ELEMENT
      C

```

ASHS0737
 ASHS0738
 ASHS0739
 ASHS0740
 ASHS0741
 ASHS0742
 ASHS0743
 ASHS0744
 ASHS0745
 ASHS0746
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 ASHS0748
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 ASHS0756
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 ASHS0768
 ASHS0769
 ASHS0770
 ASHS0771
 ASHS0772
 ASHS0773
 ASHS0774
 ASHS0775
 ASHS0776
 ASHS0777
 ASHS0778

```

C      NR=(KD-1)*N+1
      NH=NR+N-1
      DO 115 K=NR,NH
115   A(K)=A(K)/D
C
C      REDUCE REMAINING ROWS AND COLUMNS
C
      L=1
      DO 135 J=1,NN
      IF (J-KD) 130,125,130
125   L=L+N
      GO TO 135
130   DO 134 K=NR,NH
      A(L)=A(L)-C(J)*A(K)
134   L=L+1
135   CONTINUE
C
C      REDUCE ROW
C
      C(KD)=-1.0
      J=KD
      DO 140 K=1,NN
      A(J)=-C(K)/D
140   J=J+N
C
C      INTERCHANGE COLUMNS
C
      DO 200 I=1,NN
      L=0
150   L=L+1
      IF(M(L)-I) 150,160,150
160   K=(L-1)*N+1
      J=(I-1)*N+1
      M(L)=M(I)
      M(I)=I
      DO 200 L=1,NN
      TEMP=A(K)
      A(K)=A(J)
      A(J)=TEMP
      J=J+1
200   K=K+1
  
```


ASHS0779
ASHS0780
ASHS0781
ASHS0782

RETURN

END

C

C

```

C
SUBROUTINE PRINTM (A,NR,NC,MAXR)
SUBROUTINE TO PRINT ANY ARRAY
DIMENSION A(1),NHED(10)
COMMON KINN,KOUT
C
DO 50 I=1,NC,10
  II=NC-I+1
  IF (II-10) 20,20,10
  10 II=10
  20 DO 30 J=1,II
  30 NHED(J)=I+J-1
C
WRITE OUTPUT TAPE KOUT,120,(NHED(J),J=1,II)
C
DO 50 J=1,NR
  KL=J+(I-1)*MAXR
  KH=KL+(II-1)*MAXR
  50 WRITE OUTPUT TAPE KOUT,130,(J,(A(K),K=KL,KH,MAXR))
C
RETURN
C
120 FORMAT (8H0 N/M 10I11)
130 FORMAT (15,3X,10F11.3)
END

```

```

ASHS0839
ASHS0840
ASHS0841
ASHS0842
ASHS0843
ASHS0844
ASHS0845
ASHS0846
ASHS0847
ASHS0848
ASHS0849
ASHS0850
ASHS0851
ASHS0852
ASHS0853
ASHS0854
ASHS0855
ASHS0856
ASHS0857
ASHS0858
ASHS0859
ASHS0860
ASHS0861
ASHS0862

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APPENDIX F

PROGRAM LISTING - NON-AXISYMMETRIC HEAT SHIELDS

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CNAHS    NON-AXISYMMETRIC ANALYSIS HEAT SHIELDS
C
    DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
    1 XX(5),YY(5),S(15,15),P(15),KODE(30,10),
    2 SS(720,30),RR(720),NTAG(3),C(9,9),B(9,9),G(9,9)
    COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
    1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G,KODE,RR,SS
C
C
    50 KINN=5
    KOUT=6
    REWIND 20
C
    CALL INPUT
C
    MX=3*MMAX
    NS=NMAX*MMAX
    MM=MMAX-1
    NN=NMAX-1
    MBAND=3*(MMAX+2)
    NEQ=3*NS
    MB=MB-1
    MCC=MB+1
C
C
    READ IN ADDITIONAL BOUNDARY CONDITIONS
C
    DO 60 N=1,NMAX
    DO 60 M=1,MMAX
    60 KODE(N,M)=0
    WRITE OUTPUT TAPE KOUT,2000
    DO 70 K=1,NB
    READ INPUT TAPE KINN, 1000, N,M,KK
    KODE(N,M)=KK
    WRITE OUTPUT TAPE KOUT,2001, N,M,KODE(N,M)
    70 CONTINUE
C
    SOLVE STRUCTURE FOR EACH HARMONIC
C
    DO 500 L=1,NH
    WRITE OUTPUT TAPE KOUT,2006, L
C
    COMPUTE FOURIER COEFFICIENTS FOR HARMONIC L
C
    XL=0.0

```

```

TT=0.0
IF(L-1) 102,101,102
101 TT=TE
102 DO 105 N=1,NMAX
TC(N,MMAX)=TT
TC(N,MB)=E(1,L)+XL*(E(2,L)+XL*(E(3,L)+XL*(E(4,L)+XL*(E(5,L)
1 +XL*(E(6,L))))))
XL=XL+SQRTF((R(N+1,MB)-R(N,MB))*2+(Z(N+1,MB)-Z(N,MB))*2)
DO 104 M=1,MBB
104 TC(N,M)=TC(N,MB)
TXX=(TC(N,MMAX)-TC(N,MB))/FLOATF(MMAX-MB)**2
DO 105 M=MCC,MMAX
105 TC(N,M)=TC(N,MB)+TXX*FLOATF(M-MB)**2
C
IF(L-1) 106,106,108
106 DO 107 N=1,NMAX
DO 107 M=1,MMAX
107 TC(N,M)=TC(N,M)-TTT
108 CALL PRINTM(TC,NMAX,MMAX,30)
C
FORMATION OF STIFFNESS ARRAY
C
DO 175 I=1,NEQ
DO 170 J=1,MBAND
170 SS(I,J)=0.0
175 RR(I)=0.0
C
DO 200 N=1,NN
DO 200 M=1,MM
C
DO 180 I=1,15
P(I)=0.0
DO 180 J=1,15
180 S(I,J)=0.0
C
XX(1)=R(N,M)
XX(2)=R(N,M+1)
XX(3)=R(N+1,M)
XX(4)=R(N+1,M+1)
YY(1)=Z(N,M)
YY(2)=Z(N,M+1)
YY(3)=Z(N+1,M)
YY(4)=Z(N+1,M+1)
C

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```

C      CALL STIFFT(1,4,2)
C      CALL STIFFT(1,3,4)
C      ADD QUADRILATERAL STIFFNESS TO TOTAL STIFFNESS
C
    DO 200 I=1,6
      II=((N-1)*MMAX+M-1)*3+I
      KK=II+MX
      RR(II)=RR(II)+P(I)
      RR(KK)=RR(KK)+P(I+6)
      DO 190 J=I,6
        JJ=J-I+1
        SS(II,JJ)=SS(II,JJ)+S(I,J)
        190 SS(KK,JJ)=SS(KK,JJ)+S(I+6,J+6)
      DO 200 J=1,6
        JJ=J-I+1+MX
        200 SS(II,JJ)=SS(II,JJ)+S(I,J+6)
C      STIFFNESS OF PLATE ELEMENTS
C
      MBB=MB-1
      DO 600 N=1,NN
      DO 600 M=1,MB,MBB
C      CALL STIFFP
C
      DO 600 I=1,3
        II=((N-1)*MMAX+M-1)*3+I
        KK=II+MX
        RR(II)=RR(II)+P(I)
        RR(KK)=RR(KK)+P(I+3)
        DO 590 J=I,3
          JJ=J-I+1
          SS(II,JJ)=SS(II,JJ)+S(I,J)
          590 SS(KK,JJ)=SS(KK,JJ)+S(I+3,J+3)
        DO 600 J=1,3
          JJ=J-I+1+MX
          600 SS(II,JJ)=SS(II,JJ)+S(I,J+3)
C      SET BOUNDARY CONDITIONS ALONG AXIS FOR HARMONIC L
C
      DO 208 M=1,MMAX
      IF(L-2) 205,206,207
      205 KODE(1,M)=4
  
```

```

GO TO 208
206 KODE(1,M)=3
GO TO 208
207 KODE(1,M)=7
208 CONTINUE

      MODIFY STIFFNESS MATRIX

      K=1
      DO 300 N=1,NMAX
      DO 300 M=1,MMAX
      NTAG(1)=0
      NTAG(2)=0
      NTAG(3)=0
      KK=KODE(N,M)
      IF(KK) 250,250,210
210 GO TO (230,220,245,215,225,240,235),KK
215 NTAG(1)=K
220 NTAG(2)=K+1
GO TO 250
225 NTAG(3)=K+2
230 NTAG(1)=K
GO TO 250
235 NTAG(1)=K
240 NTAG(2)=K+1
245 NTAG(3)=K+2
250 K=K+3
DO 300 LL=1,3
  II=NTAG(LL)
  IF(II)300,300,260
260 DO 270 J=1,MBAND
  SS(II,J)=0.0
  III=II+1-J
  IF(III) 270,270,265
265 SS(III,J)=0.0
270 CONTINUE
  SS(II,1)=1.0
  RR(II)=0.0
300 CONTINUE

      SOLVE FOR DISPLACEMENTS

      CALL SYMSOL(SS,RR,NEQ,MBAND)

```

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NAHS0191

```
C      CALL STRESS
C      500 CONTINUE
C      CALL OUTPUT
C      FORMAT STATEMENTS
C      1000 FORMAT (3I5)
C      2000 FORMAT (20H1BOUNDARY CONDITIONS // 18H
C      2001 FORMAT (3I6)
C      2006 FORMAT (3HIL=I3)
C      END
      N      M      CODE )
```


NAHS0192	SUBROUTINE INPUT
NAHS0193	
NAHS0194	COMMON AND DIMENSION STATEMENTS
NAHS0195	
NAHS0196	DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
NAHS0197	1 RR(10),ZZ(10),TA(30),TT(50),A(50,10),B(10,10),D(10,10),
NAHS0198	2 EE(50,10),HED(12)
NAHS0199	
NAHS0200	COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
NAHS0201	1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,RR,ZZ,TA,TT,A,B,D,EE,HED
NAHS0202	
NAHS0203	READ AND PRINT INPUT DATA
NAHS0204	
NAHS0205	READ INPUT TAPE KINN,1000,
NAHS0206	1 HED,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,ESH,XSH,ASH,TSH
NAHS0207	WRITE OUTPUT TAPE KOUT,2000, HED,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT
NAHS0208	WRITE OUTPUT TAPE KOUT,2006
NAHS0209	WRITE OUTPUT TAPE KOUT,2008, ES,XS,AS,TS
NAHS0210	WRITE OUTPUT TAPE KOUT,2007
NAHS0211	WRITE OUTPUT TAPE KOUT,2008, ESH,XSH,ASH,TSH
NAHS0212	READ INPUT TAPE KINN,1001,
NAHS0213	1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)
NAHS0214	WRITE OUTPUT TAPE KOUT,2001,
NAHS0215	1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)
NAHS0216	
NAHS0217	LEAST SQUARE EVALUATION OF MATERIAL PROPERTIES
NAHS0218	
NAHS0219	READ INPUT TAPE KINN,1002,
NAHS0220	1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)
NAHS0221	WRITE OUTPUT TAPE KOUT, 2011
NAHS0222	WRITE OUTPUT TAPE KOUT,2002,
NAHS0223	1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)
NAHS0224	
NAHS0225	DO 50 I=1,NMT
NAHS0226	A(I,1)=1.0
NAHS0227	A(I,2)=TT(I)
NAHS0228	A(I,3)=TT(I)*TT(I)
NAHS0229	A(I,4)=A(I,3)*TT(I)
NAHS0230	50 A(I,5)=A(I,4)*TT(I)
NAHS0231	
NAHS0232	CALL LEAST(A,EE,B,D,CC,NMT,5,3)
NAHS0233	
NAHS0234	WRITE OUTPUT TAPE KOUT, 2012
NAHS0235	WRITE OUTPUT TAPE KOUT,2002,

1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)

GENERATE MESH

MC = MB+1

MBB = MB-1

SHL = FLOATF(MBB)

ABL = FLOATF(MMAX-MB)

DO 200 N=1,NMAX

CHECK FOR END POINTS

NL = N-1

NNH=N+1

IF(NL)160,150,160

150 SS = 0.

CX = 1.

GO TO 185

160 IF(N-NMAX)180,170,180

170 NNH=N

COMPUTE SIN AND COS AT EACH POINT ON BOND LINE

180 XX = R(NNH,MB) - R(NL,MB)

YY = Z(NNH ,MB) - Z(NL,MB)

ZZ = SQRTF(XX**2 + YY**2)

SS = YY/ZZ

CX = XX/ZZ

COMPUTE COORDINATES OF POINTS IN CONSTANT THICKNESS SHELL

185 DO 190 M=1,MBB

TT = TS*FLOATF(MB-M)/SHL

T(N,M) = T(N,MB)

R(N,M) = R(N,MB) + TT*SS

190 Z(N,M) = Z(N,MB) - TT*CX

COMPUTE COORDINATES OF POINTS IN VARIABLE THICKNESS ABLATER

DO 200 M=MC,MMAX

TT = TA(N)*FLOATF(M-MB)/ABL

T(N,M) = T(N,MB) + (TE - T(N,MB)) * (TT/TA(N))**2

R(N,M) = R(N,MB) - TT*SS

200 Z(N,M) = Z(N,MB) + TT*CX

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C      DUMP OUT R-ORDINATE TABLE AND Z-ORDINATE TABLE
C
C      WRITE OUTPUT TAPE KOUT,2003
C      CALL PRINTM (R,NMAX,MMAX,30)
C      WRITE OUTPUT TAPE KOUT,2004
C      CALL PRINTM (Z,NMAX,MMAX,30)
C      WRITE OUTPUT TAPE KOUT,2005
C      CALL PRINTM (T,NMAX,MMAX,30)
C
C      READ IN THREE-DIMENSIONAL TEMPERATURE DISTRIBUTION
C
C      TT(1)=0.0
C      DO 240 I=2,19
C      240 TT(I)=TT(I-1)+10.
C      READ INPUT TAPE KINN, 1003,((RR(J),J=1,9),(ZZ(J),J=1,9))
C      WRITE OUTPUT TAPE KOUT,2009,((RR(J),J=1,9),(ZZ(J),J=1,9))
C      READ INPUT TAPE KINN, 1003,((EE(I,J),J=1,9),I=1,19)
C      WRITE OUTPUT TAPE KOUT, 2013
C      WRITE OUTPUT TAPE KOUT,2010,(TT(I),(EE(I,J),J=1,9),I=1,19)
C
C      DO 250 I=1,19
C      XX=0.17453*FLOATF(I-1)
C      DO 250 J=1,NH
C      YY=XX*FLOATF(J-1)
C      250 A(I,J)=COSF(YY)
C
C      DO 270 I=1,9
C      DO 260 J=1,NH
C      E(J,I)=.5*(EE(1,I)*A(1,J)+EE(19,I)*A(19,J))
C      DO 255 K=2,18
C      255 E(J,I)=E(J,I)+EE(K,I)*A(K,J)
C      260 E(J,I)=E(J,I)/9.
C      270 E(1,I)=E(1,I)/2.
C
C      DO 280 I=1,19
C      DO 280 J=1,9
C      EE(I,J)=0.0
C      DO 280 K=1,NH
C      280 EE(I,J)=EE(I,J)+A(I,K)*E(K,J)
C
C      WRITE OUTPUT TAPE KOUT, 2014
C      WRITE OUTPUT TAPE KOUT,2010,(TT(I),(EE(I,J),J=1,9),I=1,19)
C
C

```

DETERMINE FOURIER COEFFICIENTS AS A FUNCTION OF SPACE

```

C
C
  XL=0.0
  DO 300 I=1,9
    A(I,1)=1.0
    A(I,2)=XL
    A(I,3)=XL**2
    A(I,4)=A(I,3)*XL
    A(I,5)=A(I,4)*XL
    A(I,6)=A(I,5)*XL
    XL=XL+SQRTF((RR(I+1)-RR(I))**2+(ZZ(I+1)-ZZ(I))**2)
    DO 300 J=1,NH
      300 EE(I,J)=E(J,I)
C
    CALL LEAST(A,EE,B,D,E,9,6,NH)
C
    RETURN
C
    FORMAT STATEMENTS
C
    1000 FORMAT (12A6/ 6I5,2F10.2/ 4F10.2/4F10.2)
    1001 FORMAT (4F10.2)
    1002 FORMAT (4F10.2)
    1003 FORMAT (9F8.0)
    2000 FORMAT ( 1H1 12A6/
      1 33HNUMBER OF POINTS ALONG LENGTH--- I3/
      2 33H NUMBER OF POINTS THRU THICKNESS- I3/
      3 33H LOCATION OF BOND LINE----- I3/
      4 33H NUMBER OF PROPERTY CARDS----- I3/
      5 33H NUMBER OF HARMONICS----- I4/
      6 33H NUMBER OF BOUNDARY CONDITIONS--- I4/
      7 33H SURFACE TEMPERATURE OF ABLATOR-- F6.0/
      8 33H ZERO STRESS TEMPERATURE----- F6.0)
    2001 FORMAT (15H1 R-ORDINATE 5X 10HZ-ORDINATE 5X 10HBOND TEMP. 3X
      1 17HABLATOR THICKNESS / (3F15.3,1F20.4))
    2002 FORMAT (15H0 TEMPERATURE 5X 10HMODULUS A 5X 9HMODULUS B 1X
      120H COEFF. OF EXPANSION / (3F15.0,1E20.5))
    2003 FORMAT (14H1 R-ORDINATES )
    2004 FORMAT (14H1 Z-ORDINATES )
    2005 FORMAT (14H1 TEMPERATURE )
    2006 FORMAT (37H0PROPERTIES OF SANDWICH CORE MATERIAL )
    2007 FORMAT (30H0PROPERTIES OF SANDWICH PLATES )
    2008 FORMAT
      1(28H MODULUS OF ELASTICITY----- F10.0/

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2	28H	POISSONS RATIO	-----	F10.4/	NAHS0368
3	28H	COEFFICIENT OF EXPANSION	--	F10.8/	NAHS0369
4	28H	THICKNESS	-----	F10.4)	NAHS0370
2009	FORMAT	(5H1 R=9F12.4/5H Z=9F12.4)			NAHS0371
2010	FORMAT	(/(F5.0.9F12.1))			NAHS0372
2011	FORMAT	(20H1 MATERIAL PROPERTIES)			NAHS0373
2012	FORMAT	(38H1 LEAST SQUARE EVALUATION OF ABOVE DATA)			NAHS0374
2013	FORMAT	(36H0 ANGLE BOND LINE TEMPERATURES)			NAHS0375
2014	FORMAT	(1H010X.47H FOURIER SERIES EXPANSION OF ABOVE TEMPERATURES)			NAHS0376
	END				NAHS0377

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B(1,2)=EX
 B(1,3)=EX
 B(2,1)=EX
 B(2,2)=DX
 B(2,3)=EX
 B(3,1)=EX
 B(3,2)=EX
 B(3,3)=DX
 B(4,4)=CX
 B(5,5)=CX
 B(6,6)=CX

FORM G MATRIX

D=XX(JJ)*(YY(KK)-YY(II))+XX(II)*(YY(JJ)-YY(KK))
 1 +XX(KK)*(YY(II)-YY(JJ))
 XN=L-1

G(1,1)=(YY(JJ)-YY(KK))/D
 G(1,4)=(YY(KK)-YY(II))/D
 G(1,7)=(YY(II)-YY(JJ))/D
 G(2,1)=1./(XX(II)+XX(JJ)+XX(KK))
 G(2,4)=G(2,1)
 G(2,7)=G(2,1)
 G(2,2)=XN*G(2,1)
 G(2,5)=XN*G(2,4)
 G(2,8)=XN*G(2,7)
 G(3,3)=(XX(KK)-XX(JJ))/D
 G(3,6)=(XX(II)-XX(KK))/D
 G(3,9)=(XX(JJ)-XX(II))/D
 G(4,1)=-G(2,2)
 G(4,4)=-G(2,5)
 G(4,7)=-G(2,8)
 G(4,2)=G(1,1)-G(2,1)
 G(4,5)=G(1,4)-G(2,4)
 G(4,8)=G(1,7)-G(2,7)
 G(5,1)=G(3,3)
 G(5,4)=G(3,6)
 G(5,7)=G(3,9)
 G(5,3)=G(1,1)
 G(5,6)=G(1,4)
 G(5,9)=G(1,7)
 G(6,2)=G(3,3)
 G(6,5)=G(3,6)

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G(6,8)=G(3,9)
G(6,3)=G(4,1)
G(6,6)=G(4,4)
G(6,9)=G(4,7)
C
C
C
FORM RING STIFFNESS MATRIX (A)T(B)(A)
C
DO 150 I=1,6
DO 150 J=1,9
C(I,J)=0.0
DO 150 K=1,6
150 C(I,J)=C(I,J)+B(I,K)*G(K,J)
C
COMM=RBAR*AREA
DO 165 I=1,9
DO 165 J=1,9
B(I,J)=0.0
DO 160 K=1,6
160 B(I,J)=B(I,J)+G(K,I)*C(K,J)
165 B(I,J)=B(I,J)*COMM
C
C
C
COMPUTE TEMPERATURE LOADS
TEM=(TC(N,M)+TC(N,M+1))+TC(N+1,M)+TC(N+1,M+1))/4.
TTT=TEM*EP*ALP/(1.-2.*XNU)
TEM=TTT*RBAR*AREA
DO 170 I=1,9
170 EE(I)=(G(1,I)+G(2,I)+G(3,I))*TEM
C
C
C
ADD TRIANGULAR RING STIFFNESS TO QUADRILATERAL RING STIFFNESS
C
C
C
LM(1)=3*I1-3
LM(2)=3*JJ-3
LM(3)=3*KK-3
DO 250 I=1,3
DO 250 K=1,3
II=LM(I)+K
KK=3*I1-3+K
P(II)=P(II)+EE(KK)
DO 250 J=1,3
DO 250 LX=1,3
JJ=LM(J)+LX
LL=3*J-3+LX
250 S(II,JJ)=S(II,JJ)+B(KK,LL)

```


NAHS0510
NAHS0511
NAHS0512
NAHS0513

RETURN

END

C C

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SUBROUTINE STIFFP

 DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
 1 XX(5),YY(5),S(15,15),P(15),G(9,9),B(9,9),C(9,9)
 COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
 1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G

 FORMATION OF G MATRIX

 XN=L-1
 A=R(N+1,M)-R(N,M)
 B=Z(N+1,M)-Z(N,M)
 XL2=A**2+B**2
 XL=SQRTF(XL2)
 RBAR=R(N+1,M)+R(N,M)

 G(1,1)=-A/XL2
 G(1,2)=0.0
 G(1,3)=-B/XL2
 G(1,4)=-G(1,1)
 G(1,5)=0.0
 G(1,6)=-G(1,3)
 G(2,1)=1./RBAR
 G(2,2)=XN*G(2,1)
 G(2,3)=0.0
 G(2,4)=G(2,1)
 G(2,5)=G(2,2)
 G(2,6)=0.0
 G(3,1)=-XN*A/RBAR/XL
 G(3,2)=1./XL
 G(3,3)=-XN*B/RBAR/XL
 G(3,4)=G(3,1)
 G(3,5)=-G(3,2)
 G(3,6)=G(3,3)

 FORMATION OF B MATRIX

 B(1,1)=ESH/(1.-XSH**2)
 B(1,2)=XSH*B(1,1)
 B(1,3)=0.0
 B(2,1)=B(1,2)
 B(2,2)=B(1,1)
 B(2,3)=0.0
 B(3,1)=0.0

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      B(3,2)=0.0
      B(3,3)=.5*ESH/(1.+XSH)
C
C      FORMATION OF PLATE STIFFNESS MATRIX (G)T*(B)*(G)
C
      DO 150 I=1,3
      DO 150 J=1,6
      C(I,J)=0.0
      DO 150 K=1,3
      150 C(I,J)=C(I,J)+B(I,K)*G(K,J)
C
      COMM=RBAR*XL*TSH/2.
      DO 165 I=1,6
      DO 165 J=1,6
      S(I,J)=0.0
      DO 160 K=1,3
      160 S(I,J)=S(I,J)+G(K,I)*C(K,J)
      165 S(I,J)=S(I,J)*COMM
C
C      COMPUTE TEMPERATURE LOADS
C
      TEM=(TC(N,M)+TC(N+1,M))/2.
      TTT=TEM*ESH*ASH*(1.+XSH)/(1.-XSH**2)
      TEM=TTT*COMM
      DO 170 I=1,6
      170 P(I)=(G(1,I)+G(2,I))*TEM
C
      RETURN
C
      END
  
```

SUBROUTINE STRESS
 DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
 1 XX(5),YY(5),S(15,15),P(15),U(30,8),V(30,8),W(30,8),KODE(30,10),
 2 C(9,9),RR(720),SIG(6,30,8),PSIG(3,30,2),B(9,9),G(9,9)
 COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
 1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G,KODE,RR,U,V,W,SIG,
 3 PSIG
 C
 C
 C
 SEPARATE DISPLACEMENTS FOR HARMONIC L
 K=1
 DO 400 N=1,NMAX
 DO 400 M=1,MMAX
 U(N,M)=RR(K)
 V(N,M)=RR(K+1)
 W(N,M)=RR(K+2)
 400 K=K+3
 C
 C
 C
 CALCULATION OF STRESSES FOR HARMONIC L
 NN=NMAX-1
 MM=MMAX-1
 DO 100 N=1,NN
 N=N
 DO 100 M=1,MM
 M=M
 XX(1)=R(N,M)
 XX(2)=R(N,M+1)
 XX(3)=R(N+1,M)
 XX(4)=R(N+1,M+1)
 YY(1)=Z(N,M)
 YY(2)=Z(N,M+1)
 YY(3)=Z(N+1,M)
 YY(4)=Z(N+1,M+1)
 CALL STIFFT(1,4,2)
 C
 C
 P(1)=U(N,M)
 P(2)=V(N,M)
 P(3)=W(N,M)
 P(4)=U(N+1,M+1)
 P(5)=V(N+1,M+1)
 P(6)=W(N+1,M+1)
 P(7)=U(N,M+1)

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DO 200 I=1,3
  PSIG(I,N,K)=0.0
  DO 200 J=1,6
    200 PSIG(I,N,K)=PSIG(I,N,K)+C(I,J)*P(J)
    PSIG(1,N,K)=PSIG(1,N,K)-TTT
    210 PSIG(2,N,K)=PSIG(2,N,K)-TTT

  WRITE TAPE 20, ((( PSIG(I,N,K),I=1,3),K=1,2),N=1,NN)

  RETURN

  END
  
```

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SUBROUTINE OUTPUT
  DIMENSION U(30,8),V(30,8),W(30,8),XU(30,8),XV(30,8),XW(30,8),
  1 SIG(6,30,8),XSIG(6,30,8),PSIG(3,30,2),XPSIG(3,30,2)
  COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
  1 U,V,W,XU,XV,XW,SIG,XSIG,PSIG,XPSIG
  NN=NMAX-1
  MM=MMAX-1
  50 READ INPUT TAPE KINN,1000,THETA
  DO 110 N=1,NMAX
  DO 100 M=1,MMAX
  U(N,M)=0.0
  V(N,M)=0.0
  W(N,M)=0.0
  DO 100 I=1,6
  100 SIG(I,N,M)=0.0
  DO 110 K=1,2
  DO 110 I=1,3
  110 PSIG(I,N,K)=0.0
  C
  REWIND 20
  DO 500 LL=1,NH
  TT=FLOATF(LL-1)*THETA*.017453289
  COSINE=COSF(TT)
  SINE=SINF(TT)
  C
  READ TAPE 20,((XU(N,M),XW(N,M),XV(N,M),(XSIG(I,N,M),I=1,6),
  1 N=1,NMAX),M=1,MMAX)
  C
  DO 200 N=1,NMAX
  DO 200 M=1,MMAX
  U(N,M)=U(N,M)+XU(N,M)*COSINE
  W(N,M)=W(N,M)+XW(N,M)*COSINE
  V(N,M)=V(N,M)+XV(N,M)*SINE
  DO 150 I=1,3
  150 SIG(I,N,M)=SIG(I,N,M)+XSIG(I,N,M)*COSINE
  SIG(4,N,M)=SIG(4,N,M)+XSIG(4,N,M)*SINE
  SIG(5,N,M)=SIG(5,N,M)+XSIG(5,N,M)*COSINE
  200 SIG(6,N,M)=SIG(6,N,M)+XSIG(6,N,M)*SINE
  C
  READ TAPE 20, (((XPSIG(I,N,K),I=1,3),K=1,2),N=1,NN)
  
```

```

C      DO 300 N=1,NN
      DO 300 K=1,2
      PSIG(1,N,K)=PSIG(1,N,K)+XPSIG(1,N,K)*COSINE
      PSIG(2,N,K)=PSIG(2,N,K)+XPSIG(2,N,K)*COSINE
      300 PSIG(3,N,K)=PSIG(3,N,K)+XPSIG(3,N,K)*SINE
C
      500 CONTINUE
C
      OUTPUT DISPLACEMENTS AND STRESSES
C
      WRITE OUTPUT TAPE KOUT,2003, THETA
      CALL PRINTM(U,NMAX,MMAX,30)
      WRITE OUTPUT TAPE KOUT,2004, THETA
      CALL PRINTM(W,NMAX,MMAX,30)
      WRITE OUTPUT TAPE KOUT,2005, THETA
      CALL PRINTM(V,NMAX,MMAX,30)
C
      DO 600 M=1,MM
      WRITE OUTPUT TAPE KOUT, 2007, THETA
      600 WRITE OUTPUT TAPE KOUT,2000, (N,M,(SIG(I,N,M),I=1,6),N=1,NN)
C
      WRITE OUTPUT TAPE KOUT, 2008, THETA
      WRITE OUTPUT TAPE KOUT,2006,(N,((PSIG(I,N,K),I=1,3),K=1,2),N=1,NN)
C
      GO TO 50
C
      1000 FORMAT (F5.0)
      2000 FORMAT (12H0      N      M 10X,2HRR 10X,2HTT 10X,2HZZ 10X,2HRT
      1 10X,2HRZ 10X,2HTZ / (2I6,6F12.2))
      2003 FORMAT (20H1R-DISPLACEMENTS AT F4.0,8H DEGREES)
      2004 FORMAT (20H1Z-DISPLACEMENTS AT F4.0,8H DEGREES)
      2005 FORMAT (20H1T-DISPLACEMENTS AT F4.0,8H DEGREES)
      2006 FORMAT (15,6F18.1)
      2007 FORMAT (28H1AVERAGE ELEMENT STRESSES AT F5.0,8H DEGREES)
      2008 FORMAT (44H1AVERAGE STRESSES IN SANDWICH FACE PLATES ATF5.0,8H DEG
      1 REES//22X, 12HBOTTOM PLATE 44X, 9HTOP PLATE /5H      N 2(54H MERN
      11D. STRESS      HOOP STRESS      IN-PLANE SHEAR ))
C
      END

```


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C      SUBROUTINE INVERT(A,NN,N,M,C)
C
C      GENERAL MATRIX INVERSION SUBROUTINE
C
C      DIMENSION A(1),M(1),C(1)
C
C      CHECK FOR 1 X 1 MATRIX
C
C      IF(NN-1) 300,70,80
70  A(1)=1./A(1)
C      GO TO 300
C
C      80 DO 90 I=1,NN
90  M(I)=-I
C
C      DO 140 I=1,NN
C      LOCATE LARGEST ELEMENT
C
C      D=0.0
C      DO 112 L=1,NN
C      IF (M(L)) 100,100,112
100  J=L
C      DO 110 K=1,NN
C      IF (M(K)) 103,103,108
103  IF (ABSF(D)-ABSF(A(J))) 105,105,108
105  LD=L
C      KD=K
C      D=A(J)
C      J=J+N
108  CONTINUE
110  CONTINUE
112  CONTINUE
C
C      INTERCHANGE ROWS
C
C      TEMP=-M(LD)
C      M(LD)=M(KD)
C      M(KD)=TEMP
C      L=LD
C      K=KD
C      DO 114 J=1,NN
C      C(J)=A(L)
C      A(L)=A(K)
C      A(K)=C(J)

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L=L+N
114 K=K+N
C
C
C
DIVIDE COLUMN BY LARGEST ELEMENT
NR=(KD-1)*N+1
NH=NR+N-1
DO 115 K=NR,NH
115 A(K)=A(K)/D
C
C
C
REDUCE REMAINING ROWS AND COLUMNS
L=1
DO 135 J=1,NN
IF (J-KD) 130,125,130
125 L=L+N
GO TO 135
130 DO 134 K=NR,NH
A(L)=A(L)-C(J)*A(K)
134 L=L+1
135 CONTINUE
C
C
C
REDUCE ROW
C(KD)=-1.0
J=KD
DO 140 K=1,NN
A(J)=-C(K)/D
140 J=J+N
C
C
C
INTERCHANGE COLUMNS
DO 200 I=1,NN
L=0
150 L=L+1
IF(M(L)-I) 150,160,150
160 K=(L-1)*N+1
J=(I-1)*N+1
M(L)=M(I)
M(I)=I
DO 200 L=1,NN
TEMP=A(K)
A(K)=A(J)
A(J)=TEMP
  
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J=J+1
200 K=K+1
C
300 RETURN
C
END

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```

C      SUBROUTINE SYMSOL (A,B,NN,MM)
C
C      DIMENSION A(720,30),B(720),C(30)
C
C      N = 0
C      100 N = N+1
C
C      REDUCE N TH EQUATION
C
C      1. DIVIDE RIGHT SIDE BY DIAGONAL ELEMENT
C
C      B(N) = B(N) / A(N,1)
C
C      2. CHECK FOR LAST EQUATION
C
C      IF(N=NN) 150,300,150
C
C      3. DIVIDE N TH EQUATION BY DIAGONAL ELEMENT
C
C      150 DO 200 K=2,MM
C      C(K) = A(N,K)
C      200 A(N,K) = A(N,K) / A(N,1)
C
C      4. REDUCE REMAINING EQUATIONS
C
C      DO 260 L=2,MM
C      I = N+L-1
C      IF(NN-I) 260,240,240
C      240 J=0
C      DO 250 K=L,MM
C      J=J+1
C      250 A(I,J) = A(I,J) - C(L) * A(N,K)
C      B(I) = B(I) - C(L) * B(N)
C      260 CONTINUE
C      GO TO 100
C
C      BACK SUBSTITUTION
C
C      300 N = N-1
C
C      1. CHECK FOR FIRST EQUATION
C
C      IF(N) 350,500,350
C
C

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C      2. CALCULATE UNKNOWN B(N)
C
      350 DO 400 K=2,MM
          L = N+K-1
          IF(NN-L) 400,370,370
      370 B(N) = B(N) - A(N,K) * B(L)
      400 CONTINUE
          GO TO 300
C
      500 RETURN
C
      END
```

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```

SUBROUTINE PRINTM (A,NR,NC,MAXR)
DIMENSION A(1),NHED(10)
COMMON KINN,KOUT
C
DO 50 I=1,NC,10
  II=NC-I+1
  IF (II-10) 20,20,10
    10 II=10
    20 DO 30 J=1,II
      30 NHED(J)=I+J-1
C
  WRITE OUTPUT TAPE KOUT,120,(NHED(J),J=1,II)
C
DO 50 J=1,NR
  KL=J+(I-1)*MAXR
  KH=KL+(II-1)*MAXR
  50 WRITE OUTPUT TAPE KOUT,130,(J,(A(K),K=KL,KH,MAXR))
C
  RETURN
C
  120 FORMAT (8H0 N/M 10I11)
  130 FORMAT (15,3X,10F11.3)
  END

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